



City of Bellevue

Transportation Analysis Report Lakemont Boulevard SE & SE Newport Way Bellevue, WA

July 2019

Contract # 1850215

PREPARED FOR



PREPARED BY

Reid Middleton

**City of Bellevue Neighborhood Congestion Reduction Program
Lakemont Blvd SE/SE Newport Way Transportation Analysis Report
Contract Number 1850215
July 2019**

The engineering material and data contained in this report were prepared under the supervision and direction of the undersigned, whose seal as registered professional engineer is affixed below.



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CONTENTS

INTRODUCTION	1
Background.....	1
Purpose	2
EXISTING CONDITIONS	3
Traffic Conditions.....	3
<i>Collision History.</i>	6
Public Outreach	7
TRAFFIC ANALYSIS	9
Modeling Methodology	9
Alternatives Analysis.....	10
Recommended Alternative	19
<i>Anticipated Benefits</i>	19
<i>The anticipated benefits of Alternative 1 are:</i>	19
<i>Challenges and Risks</i>	20
<i>Opinion of Probable Construction Costs</i>	20
<i>Coordination with WSDOT</i>	20

FIGURES

Figure 1. Vicinity Map.....	2
Figure 2. Existing Peak Hour Traffic Volumes (Lakemont Boulevard / Newport Way).....	4
Figure 3. Existing Peak Hour Traffic Volumes (Newport Way / I-90 On-Ramp).....	4
Figure 4. Lakemont Boulevard SE/SE Newport Way & SE Newport Way/I-90 On Ramp – Alternative 1.	13
Figure 5. Lakemont Boulevard SE/SE Newport Way & SE Newport Way/I-90 On Ramp – Alternative 2.	15

TABLES

Table 1. Existing Conditions – Synchro Results SE Newport Way and Lakemont Boulevard SE.....	5
Table 2. Existing Conditions – Synchro Results SE Newport Way & I-90 On-Ramp.	6
Table 3. Collision History.....	7
Table 4. Intersection Volumes.....	9
Table 5. 2035 Alternatives – Synchro & Sidra Results (Lakemont Boulevard SE / SE Newport Way).	17
Table 6. 2035 Alternatives – Synchro & Sidra Results (Newport Way / I-90 On-Ramp).....	17
Table 7. Preferred Alternatives Analysis (Lakemont / Newport Way).....	18
Table 8. Preferred Alternatives MMLOS Analysis (Lakemont / Newport Way)	19

APPENDICES

- A – Traffic Counts**
- B – Collision Data**
- C – Origin-Destination Study**
- D - Traffic Analysis**
- E – Opinion Of Probable Construction Costs**
- F– Public Comments**

INTRODUCTION

In November 2016, voters passed the Neighborhood Safety, Connectivity and Congestion Levy, which helps the city address a backlog of needs organized into the following six categories: neighborhood safety; bicycle facilities; new sidewalks; sidewalk and trail maintenance; traffic management technology; and neighborhood congestion reduction. The levy provides approximately \$2 million per year for the Neighborhood Congestion Reduction Program that focuses on reducing motor vehicle congestion for residents traveling to and from their neighborhoods. Levy funding pays for the planning, public outreach, design and construction of projects that rate the highest for reducing congestion. This study falls under the Neighborhood Congestion Reduction program.

City of Bellevue staff compiled a list of congested intersections and corridors in the city to start this program and then worked with the Transportation Commission in 2018 to develop scoring criteria to rank these projects. Nine locations were selected for evaluation in 2018. After this study is completed, the City will compare the benefits and costs of this project with other Neighborhood Congestion Reduction projects throughout the city to determine which projects will move forward to design and construction.

Background

The Lakemont Boulevard SE and SE Newport Way corridors are major and minor arterials respectively that provide access to residential neighborhoods, schools, parks, and employment centers, and travel between I-90, Cougar Mountain/Lakemont, and Issaquah.

The two existing intersections that are being evaluated for this study include:

- Lakemont Boulevard SE and SE Newport Way.
- SE Newport Way and I-90 Eastbound On-Ramp.

The signalized intersections of SE Newport Way at Lakemont Boulevard SE and the I-90 eastbound on-ramp are less than 400 feet apart. This close spacing contributes to congestion and traffic backups between intersections, particularly during peak periods. During the AM peak hour, northbound traffic on Lakemont Boulevard experiences long queues and delays along with eastbound left-turn movements to I-90 from SE Newport Way. During the PM peak hour southbound left-turn traffic from Lakemont Boulevard SE to SE Newport Way experiences excessive queueing and delays. The intersections are being evaluated to assess future capacity, level-of-service (LOS), and operational safety.

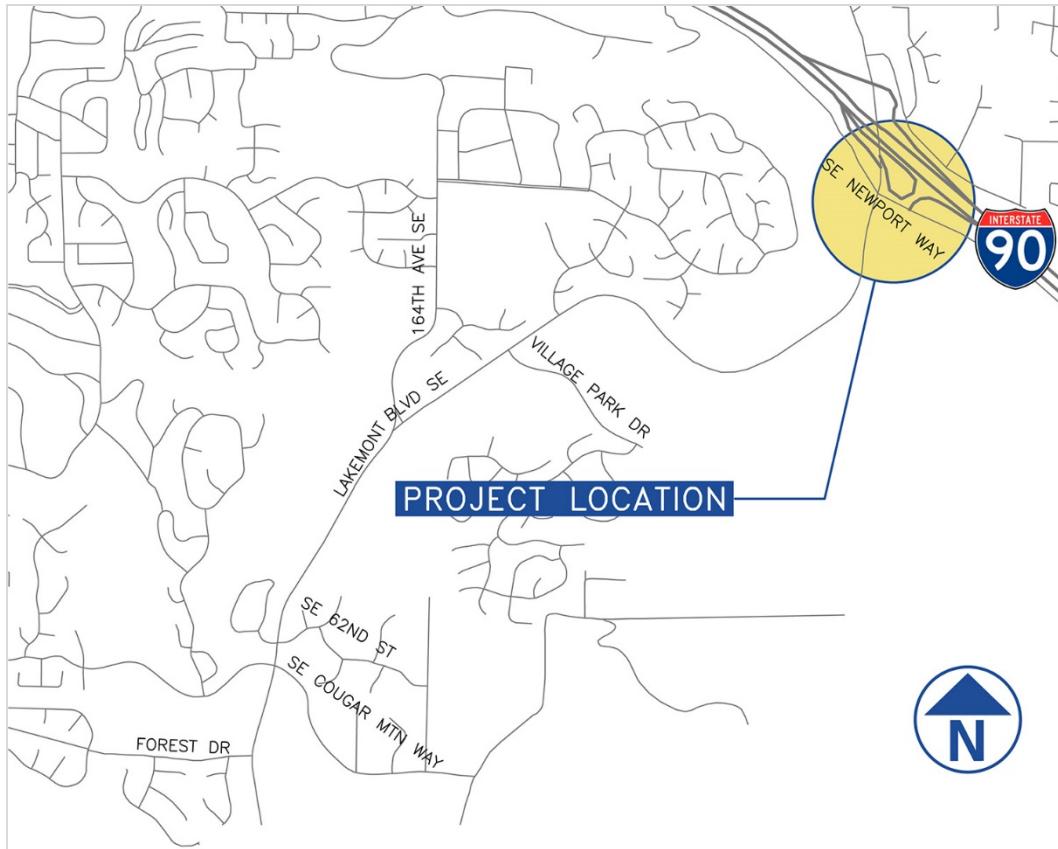


Figure 1. Vicinity Map.

Purpose

This report summarizes the analysis performed for the Lakemont Boulevard SE and SE Newport Way intersection area including the SE Newport Way/I-90 Eastbound On-ramp intersection. The study includes evaluation of existing conditions, future no-build conditions, and alternatives analysis for conceptual design alternatives for the intersections at Lakemont Boulevard SE and SE Newport Way and the I-90 Eastbound On-Ramp. The alternatives analysis includes the advantages and disadvantages of each alternative, a preliminary analysis of probable construction costs, and the assessment of multimodal and safety impacts related to each alternative. A conceptual layout of the preferred alternative is included, along with the challenges and risks associated with the proposed alternative.

EXISTING CONDITIONS

Traffic Conditions

Lakemont Boulevard SE and SE Newport Way provide access to the I-90 corridor for neighborhoods on both sides of I-90. The I-90 eastbound off-ramp splits into separate directional ramp connections to northbound and southbound lanes on Lakemont Boulevard SE that are merge- and stop-controlled, respectively. The I-90 eastbound on-ramp is accessed from a signalized T intersection on SE Newport Way east of Lakemont Boulevard SE.

Lakemont Boulevard SE consists of two northbound and two southbound lanes near its signalized intersection at SE Newport Way, with additional left- and right-turn lanes provided at the intersection. Lakemont Boulevard SE has paved shoulders on both sides of the roadway north of SE Newport Way. South of SE Newport Way, there are marked bike lanes/shoulders on both sides and a sidewalk on the west side. The posted speed on Lakemont Boulevard SE is 30 MPH north of Newport Way and 40 MPH to the south.

SE Newport Way is a two-lane roadway with additional left- and right-turn lanes provided at both the Lakemont Boulevard SE and I-90 Eastbound On-ramp intersections. There are paved shoulders on both sides of the roadway. The posted speed on SE Newport Way is 30 MPH east of Lakemont Boulevard SE and 40 MPH on the west leg.

King County Metro operates bus route 271 on the west and north legs of the Lakemont Boulevard SE/SE Newport Way intersection with all-day service (60 total trips, 30 each way).

Traffic counts were conducted in November 2018 at all project intersections (see Appendix A). Figures 2 and 3 show the existing peak hour volumes for the Lakemont Boulevard SE/SE Newport Way and SE Newport Way/I-90 Eastbound On-ramp intersections respectively.

At the intersection of Lakemont Boulevard SE and SE Newport Way the northbound and southbound volumes on Lakemont Boulevard SE are especially heavy. Traffic is predominantly traveling northbound in the morning and southbound in the afternoon.

At the intersection of SE Newport Way and the I-90 Eastbound On-ramp, eastbound left-turn volumes headed eastbound on I-90 are heavy in both AM and PM peak hours. The eastbound through volume on SE Newport Way is also heavy in the PM peak hour. The heaviest westbound volume on SE Newport Way occurs in the morning.

An origin-destination analysis was conducted using *Streetlight* data to determine how much of the Eastbound I-90 Off-ramp traffic uses either of the directional ramp connections to northbound or southbound Lakemont Boulevard to access SE Newport Way (see Appendix C). The results indicate that a significant amount of traffic uses the loop ramp connection to northbound Lakemont Boulevard to make a U-turn at the roundabout on the north side of I-90 and travel southbound on Lakemont Boulevard to turn onto SE Newport Way. The volume of traffic using this U-turn route represents approximately 34 percent of AM and 42 percent of PM peak hour loop ramp traffic. This traffic is avoiding congestion on the other directional ramp

associated with the right-turn movement onto southbound Lakemont Boulevard. More details of the origin-destination study can be found in Appendix A.

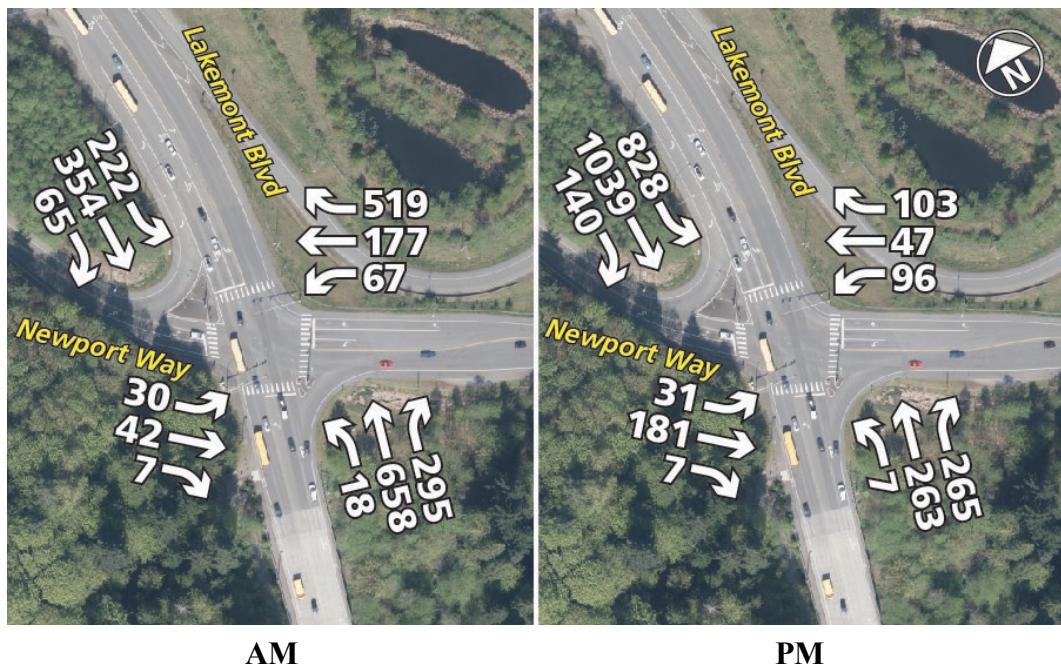


Figure 2. Existing Peak Hour Traffic Volumes (Lakemont Boulevard / Newport Way).

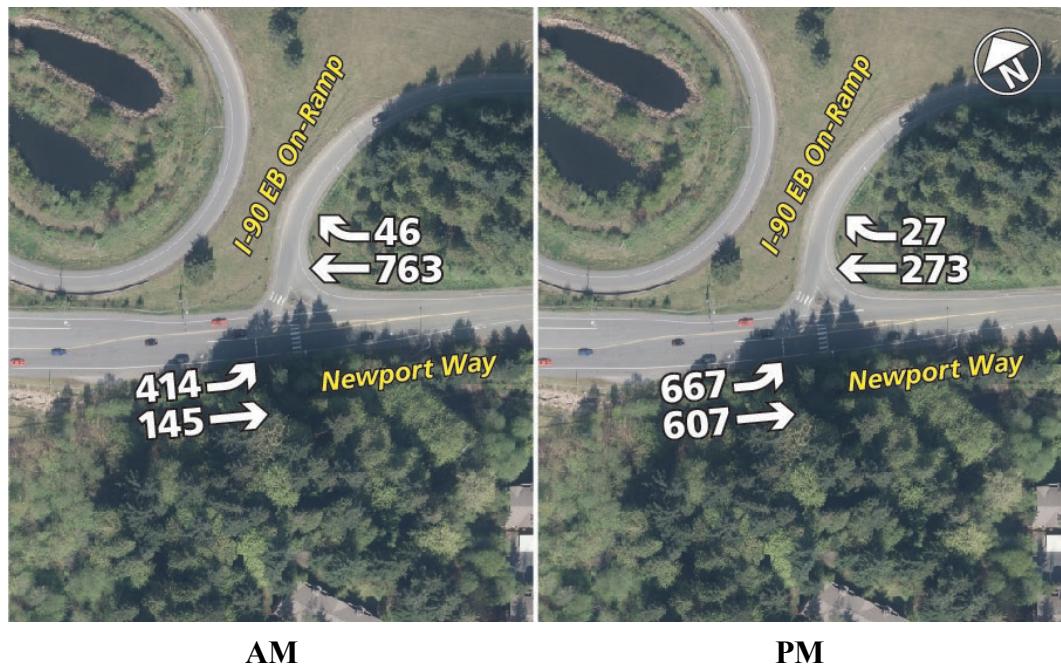


Figure 3. Existing Peak Hour Traffic Volumes (Newport Way / I-90 On-Ramp).

Existing traffic conditions at both project intersections were modeled using Synchro (see Appendix D). The results of the analysis are shown in Tables 1 and 2.

Table 1. Existing Conditions – Synchro Results SE Newport Way and Lakemont Boulevard SE.

Street	Movement	Existing Conditions (Current)			
		Delay(s)	LOS	Avg. Queue	95th Percentile Queue
AM Peak					
SE Newport Way	EBL	42	D	21	46
	EBT/R	31	C	28	53
	WBL	44	D	40	87
	WBT	36	D	105	175
	WBR	15	B	144	260
Lakemont Boulevard SE	NBL	24	C	8	27
	NBT/R	37	D	278	478
	SBL	34	C	103	200
	SBL/T or SBT	31	C	108	178
	SBR	2	A	0	7
Intersection		30	C		
PM Peak					
SE Newport Way	EBL	61	E	32	58
	EBT/R	65	E	190	237
	WBL	68	E	102	141
	WBT	41	D	41	69
	WBR	2	A	0	11
Lakemont Boulevard SE	NBL	42	D	5	19
	NBT/R	41	D	160	228
	SBL	50	D	523	846
	SBL/T or SBT	42	D	545	763
	SBR	8	A	15	60
Intersection		43	D		

Table 2. Existing Conditions – Synchro Results SE Newport Way & I-90 On-Ramp.

Street	Movement	Existing Conditions (Current)			
		Delay(s)	LOS	Avg. Queue	95th Percentile Queue
AM Peak					
SE Newport Way	EBL	24	C	75	267
	EBT	0	A	0	0
	WBT/R	14	B	237	356
Intersection		16	B		
PM Peak					
SE Newport Way	EBL	9	A	0	139
	EBT	0	A	0	0
	WBT/R	19	B	80	118
Intersection		8	A		

The results indicate the Lakemont Boulevard SE and SE Newport Way intersection is performing at LOS D or better under existing peak hour conditions; however, the split phase condition for northbound and southbound approaches on Lakemont Boulevard SE limits overall capacity of the intersection. In general, left-turn movements are functioning at worse levels of service than other movements, and the overall intersection is nearing capacity for the current PM peak hour traffic volumes.

The results for the SE Newport Way and I-90 Eastbound On-ramp intersection indicate that the intersection is performing at LOS B or better under existing peak hour conditions; however, left-turn movements are functioning at worse levels of service than the through movements.

The following observations were made during a site visit:

- Bicycle activity was observed at both intersections, and moderate levels of delay were noted.
- At the intersection of SE Newport Way and I-90 Eastbound On-Ramp, an eastbound vehicle was observed running a red light during the conflicting pedestrian phase.
- The inside eastbound through lane is utilized more than the outside eastbound through lane on SE Newport Way, due to the merge point downstream of the intersections.

Collision History

The City of Bellevue provided the raw crash data for the study area for the last five years (see Appendix B). The data were analyzed and post processed. Crashes were grouped based on different crash types and are shown in Table 3. Twenty-one collisions occurred at the

intersection of Lakemont Boulevard SE and SE Newport Way over the past five years. The majority of the collisions are rear-end crashes occurring in the northbound and southbound directions. There are about twice as many northbound rear-end collisions as there are southbound, which suggests that the grade on Lakemont Boulevard SE may be a factor.

Table 3. Collision History.

	Crash Count
Approach Turn	1
Head On	0
Other	1
Parked Vehicle/Fixed Object	2
Pedestrian	0
Rear End	11
Right Angle	3
Sideswipe/Lane Change	3
Total	21

Public Outreach

An open house for the Lakemont Boulevard SE and SE Newport Way corridors analysis was conducted on the evening of February 20, 2019. The open house was held at the Lewis Creek Park Visitor Center. Comments received included comment cards from open house participants and email correspondence from community members that were unable to attend. The following is a summary of comments received:

- Roundabouts are “a slam dunk”.
- Like having two lanes for traffic turning onto I-90 eastbound ramp.
- Some respondents did not like the idea of interruptions during construction. The interruption to traffic on Lakemont Boulevard would be undesirable.
- One respondent suggested a longer protected green for left turns on to I-90 would be sufficient.

Copies of comments received are included in Appendix F.

TRAFFIC ANALYSIS

Modeling Methodology

AM and PM peak hour traffic analyses were conducted to evaluate operational improvements at the Lakemont Boulevard SE/SE Newport Way and SE Newport Way/I-90 Eastbound On-ramp intersections. The operation analysis covered the Existing, Future No-Build, and Future Build conditions for the study area.

Synchro was used for the analysis of traffic signals alternatives and Sidra was used for the analysis of roundabout alternatives. The City of Bellevue provided the 2035 future volumes used in this analysis and shown in Table 4.

Table 4. Intersection Volumes.

Lakemont Boulevard SE & SE Newport Way	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
AM Peak	30	60	10	40	220	480	20	600	330	370	340	120
	30	42	7	67	177	519	18	658	295	222	354	65
PM Peak	30	170	0	80	50	140	10	230	270	1130	970	160
	31	181	7	96	47	103	7	263	265	828	1039	140
SE Newport Way & I-90 On-ramp	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
AM Peak	600	170	0	0	730	40	0	0	0	0	0	0
	414	145	0	0	763	46	0	0	0	--	--	--
PM Peak	1160	570	0	0	230	30	0	0	0	0	0	0
	667	607	0	0	273	27	0	0	0	--	--	--

Legend: 2035 Intersection Volumes
Existing Volumes

Alternatives Analysis

Future scenarios were considered for the two project intersections in the corridor. Alternatives analysis considered modifications to the signalized intersections as well as roundabout options for each intersection. A key assumption for implementing any improvements was that the two intersections would either both operate as signals or as roundabouts, so that if one intersection was converted to a roundabout the other would be converted as well. Each intersection has been evaluated for the following:

- Future No Build: Maintain existing configuration.
- Signalized Alternatives: Signalized improvements to the intersections.
- Roundabout Alternatives: Roundabout options for the intersections.

Synchro and Sidra analyses were performed for the two project intersections to evaluate Future No-Build, Future Modified Signal, and Future Roundabout alternatives (see Appendix D). See Tables 5 and 6 for a breakdown of the results. A full comparison of all of the alternatives considered for the project are included in Appendix A. The results show that in the future design year 2035 the Lakemont Boulevard SE/SE Newport Way and SE Newport Way/I-90 Eastbound On-ramp intersections operate at LOS D and E respectively during the PM peak hour with no improvements. However, the 95th percentile queue for the eastbound left-turn movement from SE Newport Way onto the I-90 Eastbound On-ramp backs up beyond the intersection at Lakemont Boulevard SE during both peak hours, which indicates that intersection delays at Lakemont Boulevard SE are actually worse than reported by Synchro.

Synchro analysis of various modifications of the existing signalized configuration demonstrated that converting the southbound through/left lane on Lakemont Boulevard to a second left-turn lane and similarly converting one of the eastbound through lanes on SE Newport Way to another left-turn lane at the ramp intersection provides the optimal improvement for the signal operation. A conceptual plan showing the modified signal alternative is included in Figure 4. This alternative (Alternative 1) improves the intersections operations from LOS D/E in the PM to LOS C/B in the future design year while reducing the 95th percentile queues for the eastbound left-turn on to the ramp to be accommodated between intersections. This alternative also assumes that a future connection from the I-90 Eastbound Off-ramp to SE Newport Way is not constructed.

Sidra analysis was performed for future peak hour volumes and showed the optimal roundabout configuration to be a hybrid multi-lane roundabout with right-turn bypass lanes for northbound, southbound, and westbound approaches and a two-lane tear-drop roundabout at SE Newport Way and the I-90 Eastbound On-ramp. A conceptual plan showing the roundabout alternative is included in Figure 5. This alternative (Alternative 2) improves the intersections operations to LOS A in the future design year. This alternative also assumes that a future connection from the I-90 Eastbound Off-ramp to SE Newport Way is not constructed. Although a previous study by WSDOT proposed a future connection, an evaluation of this configuration demonstrated that it would degrade the intersection to LOS F; therefore, it was not included in the future traffic projections.

It should be noted that the layout for Alternative 2 includes widening the eastbound approach of SE Newport Way at Lakemont Boulevard SE to two lanes. A one-lane eastbound approach was also modeled which resulted in a V/C of 0.90 for this approach as well as the intersection. Adding a second lane improves the V/C to 0.40 for the approach and 0.77 for the intersection; however, this approach has the lightest volumes of any of the intersection approaches. While a second lane benefits traffic operations it adds considerable cost, because it encroaches into the existing wall and steep slope on the south side of SE Newport Way.

Both Alternatives 1 and 2 involve widening the existing eastbound on-ramp to provide two receiving lanes for left-turn traffic from SE Newport Way; however, there is sufficient ramp length to merge the two lanes to one lane prior to the connection with the mainline so that no modification of the mainline connection is needed.

Modified Signal Control



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Figure 4 – Lakemont Blvd SE/SE Newport Way & SE Newport Way/I-90 EB On-Ramp – Alternative 1



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Figure 5 – Lakemont Blvd SE/SE Newport Way & SE Newport Way/I-90 EB On-Ramp – Alternative 2

Table 5. 2035 Alternatives – Synchro & Sidra Results (Lakemont Boulevard SE / SE Newport Way).

Street	Move- ment	Future No-Build				Alternative 1 – Modified Signals				Move- ment	Alternative 2 - Roundabouts			
		Delay (s)	LOS	Avg. Queue	95th % Queue	Delay (s)	LOS	Avg. Queue	95th % Queue		Delay (s)	LOS	V/C	95th % Queue
AM Peak														
SE Newport Way	EBL	43	D	22	46	35	D	17	40	EB	7.7	A	0.07	8
	EBT/R	30	C	41	71	22	C	31	55		-	-	-	-
	WBL	43	D	25	59	36	D	20	52	WB	3.2	A	0.33	59
	WBT	41	D	135	216	29	C	105	170		-	-	-	-
	WBR	12	B	114	216	15	B	120	214		-	-	-	-
Lakemont Blvd SE	NBL	24	C	9	29	24	C	9	26	NB	6.4	A	0.59	109
	NBT/R	35	C	272	446	27	C	200	351		-	-	-	-
	SBL	36	D	140	251	32	C	88	144	SB	5.5	A	0.32	58
	SBL/T	32	C	144	219	23	C	84	344		-	-	-	-
	SBR	7	A	0	45	4	A	0	30		-	-	-	-
Intersection		30	C			24	C				5.3	A	0.59	109
PM Peak														
SE Newport Way	EBL	59	E	29	57	59	E	27	58	EB	17.8	B	0.40	67
	EBT/R	60	E	162	217	56	E	148	214		-	-	-	-
	WBL	62	E	179	121	84	F	77	146	WB	3.8	A	0.13	19
	WBT	39	D	42	71	41	D	40	74		-	-	-	-
	WBR	2	A	0	11	2	A	0	11		-	-	-	-
Lakemont Blvd SE	NBL	42	D	7	25	52	D	7	28	NB	10.3	B	0.44	75
	NBT/R	36	D	126	198	33	C	131	210		-	-	-	-
	SBL	63	E	584	1025	33	C	365	506	SB	6.2	A	0.77	243
	SBL/T	50	D	589	917	25	C	481	1062		-	-	-	-
	SBR	9	A	21	74	3	A	9	47		-	-	-	-
Intersection		47	D			31	C				7.5	A	0.77	243

Table 6. 2035 Alternatives – Synchro & Sidra Results (Newport Way / I-90 On-Ramp).

Street	Move- ment	Future No-Build				Alternative 1 – Modified Signals				Move- ment	Alternative 2 - Roundabouts			
		Delay (s)	LOS	Avg. Queue	95th % Queue	Delay (s)	LOS	Avg. Queue	95th % Queue		Delay (s)	LOS	V/C	95th % Queue
AM Peak														
Newport Way	EBL	78	E	206	494	30	C	129	217	EB	5.9	A	0.29	0
	EBT	0	A	0	0	0	A	0	0		-	-	-	-
	WBT/R	15	B	215	327	18	B	242	430	WB	4.0	A	0.41	47
Intersection		38	D			21	C				5.0	A	0.41	47
PM Peak														
Newport Way	EBL	112	F	334	556	14	B	128	252	EB	5.3	A	0.67	0
	EBT	0	A	0	0	1	A	0	0		-	-	-	-
	WBT/R	18	B	66	101	21	C	71	146	WB	6.1	A	0.21	24
Intersection		67	E			11	B				5.4	A	0.67	24

Several other factors have been considered in addition to the traffic operations at the intersection. Table 7 provides a summary of various criteria for the preferred signal option (Alternative 1) and preferred roundabout option (Alternative 2). A preliminary Opinion of Probable Construction Costs for each of the preferred alternatives can be found in Appendix B.

Table 7. Preferred Alternatives Analysis (Lakemont / Newport Way)

Future No Build	Alternative 1 – Modified Signals	Alternative 2 - Roundabouts
Traffic Operations		
LOS D/E Significant queues during AM and PM Peak.	LOS C Improved operations by adding additional left turn lanes	LOS A V/C 0.77 or less for both intersections.
Traffic Safety		
Signalized operations with higher volumes likely to result in continued rear-end crashes.	Reduced congestion and queueing may reduce the number of crashes.	Reduced number of conflict points and vehicle speeds through intersection may reduce both the number of crashes and crash severity.
Right-of-Way		
None	None Much of the reconstructed area lies within WSDOT limited access area	None Much of the reconstructed area lies within WSDOT limited access area
Stormwater Impacts		
None	Increase in impervious surfacing will require stormwater mitigation. Likely stormwater treatment, possibly minor flow control.	Increase in impervious surfacing and new hard surfacing triggers stormwater treatment and flow control.
Utility Impacts		
None	Adjustment/relocation of underground utilities as needed to accommodate improvements.	Adjustment/relocation of underground utilities as needed to accommodate improvements.
Environmental Impacts		
None	Potential minor impacts to critical area may require mitigation.	Potential minor impacts to critical area may require mitigation.
Construction Costs		
None	\$3,300,000 (see Appendix E)	\$11,200,000 (see Appendix E)

Each alternative was also assessed to identify how it impacts the Multi-Modal LOS for pedestrians, bikes, and transit. Table 8 summarizes the assessment.

Table 8. Preferred Alternatives MMLOS Analysis (Lakemont / Newport Way)

Element	Alternative 1 – Modified Signals	Alternative 2 - Roundabouts
Pedestrian LOS		
Sidewalk & Landscape Buffer	Improves: Sidewalk is provided on south side of Newport Way.	Improves: Multi-use path with landscape buffer is provided within the roundabouts on the south side of Newport Way and west side of Lakemont Boulevard.
Intersection Treatment	No Change	Improves: The number of conflict points between vehicles and pedestrians is reduced.
Bicycle LOS		
Intersection Treatment	No Change	Improves: Multi-use path with bicycle ramps is provided on entrances and exits of the roundabouts where possible to give bicyclists the option of having a physical separation from vehicles.
Transit LOS		
Passenger Amenities	No Change	No change
Transit Speed	Improves: Vehicle delay is reduced, which reduces travel times for transit.	Improves: Vehicle delay is reduced, which reduces travel times for transit.

Recommended Alternative

Alternative 2, with roundabouts constructed at the intersections of Lakemont Boulevard SE/SE Newport Way and SE Newport Way/I-90 On-ramp is the preferred long-term improvement because it provides the best overall traffic operations of the alternatives and will have a calming effect on traffic. However, Alternative 1, with modifications to the existing signalized intersections, is recommended as the best near-term solution because it provides needed congestion relief at less than a third of the cost of roundabouts.

Anticipated Benefits

The anticipated benefits of Alternative 1 are:

- Reduced congestion relieves driver frustration.
- Cost effective solution for the level of investment.

Challenges and Risks

The challenges and risks associated with Alternative 1 are:

- Queues on SE Newport Way between Lakemont Boulevard and the I-90 Eastbound On-ramp will reach nearly to the upstream intersection by 2035.
- Much of the reconstruction lies within WSDOT Limited Access area, which will require considerable agency coordination.
- Impacts to critical areas are thought to be minor but further investigation is needed to confirm the level of impact.

Opinion of Probable Construction Costs

The preliminary opinion of probable construction costs for the preferred near-term improvement is \$3,300,000. This estimate includes construction costs only. No additional right-of-way is anticipated.

Coordination with WSDOT

A meeting was held with WSDOT on March 28, 2019, to present the findings of this study and determine the next steps going forward. Anticipated next steps include:

- Prepare the Intersection Control Evaluation (ICE). Most of the ICE work has been done.
- Through the ICE, determine the preferred alternative – roundabout vs. signal.
- A concept design with very specific elements would be developed for WSDOT headquarters (HQ) review. HQ review is needed because of impact to the freeway system.
- FHWA would need to review and would need a Basis of Design (BOD).
- Develop the Channelization Plan for WSDOT review and approval.
- Design approval is only good for three years.
- Document the preferred pedestrian paths to support the Mountains to Sound Greenway (MTSG) path connection to the City of Issaquah trail.
- In the ICE, address why WSDOT's proposal to split the I-90 eastbound loop ramp is not recommended.
- WSDOT noted that they would like to see the more constrained design, with only one lane in the roundabout eastbound approach and only one lane onto the I-90 Eastbound On-ramp.

APPENDICES

APPENDIX A: TRAFFIC COUNTS

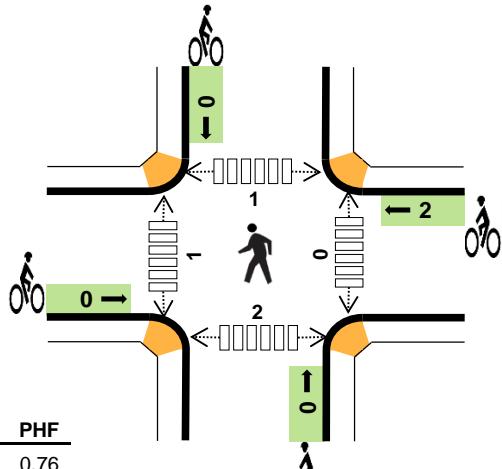
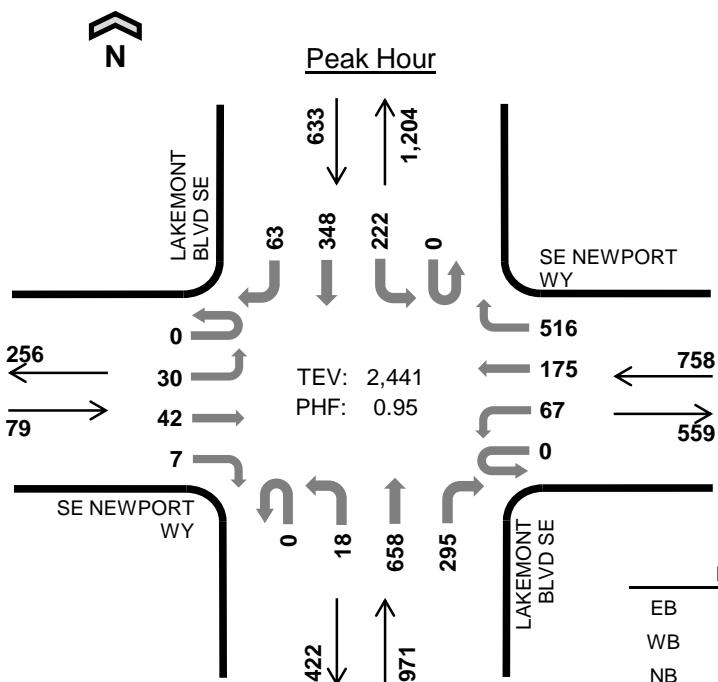
LAKEMONT BLVD SE SE NEWPORT WY



Date: Tue, Nov 13, 2018

Count Period: 7:00 AM to 9:00 AM

Peak Hour: 7:45 AM to 8:45 AM



Two-Hour Count Summaries

Interval Start	SE NEWPORT WY				SE NEWPORT WY				LAKEMONT BLVD SE				LAKEMONT BLVD SE				15-min Total	Rolling One Hour
	Eastbound		Westbound		Northbound		Southbound		UT	LT	TH	RT	UT	LT	TH	RT		
UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
7:00 AM	0	3	16	0	0	3	15	87	0	1	129	79	0	42	33	9	417	0
7:15 AM	0	7	9	0	0	18	20	109	0	2	149	92	0	51	44	10	511	0
7:30 AM	0	8	10	2	0	13	32	107	0	3	154	87	0	55	76	9	556	0
7:45 AM	0	8	8	2	0	15	51	130	0	3	138	81	0	64	77	22	599	2,083
8:00 AM	0	6	10	0	0	14	45	156	0	6	164	50	0	46	90	11	598	2,264
8:15 AM	0	10	13	3	0	22	38	117	0	3	187	76	0	61	97	14	641	2,394
8:30 AM	0	6	11	2	0	16	41	113	0	6	169	88	0	51	84	16	603	2,441
8:45 AM	0	10	13	3	0	11	36	111	0	0	114	69	0	44	84	21	516	2,358
Count Total	0	58	90	12	0	112	278	930	0	24	1,204	622	0	414	585	112	4,441	0
Peak Hour	0	30	42	7	0	67	175	516	0	18	658	295	0	222	348	63	2,441	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals				Bicycles				Pedestrians (Crossing Leg)									
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total			
7:00 AM	1	1	2	10	14	0	0	0	0	0	0	1	0	0	0	1		
7:15 AM	2	3	7	4	16	0	1	0	0	1	1	0	2	0	3			
7:30 AM	1	2	3	5	11	1	0	0	0	1	0	0	0	0	0			
7:45 AM	0	4	1	2	7	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	2	2	7	11	0	1	0	0	1	0	0	1	0	1	1	0	1
8:15 AM	1	2	3	8	14	0	1	0	0	1	0	1	0	2	3	0		
8:30 AM	2	5	3	10	20	0	0	0	0	0	0	0	0	0	0	0		
8:45 AM	1	3	2	9	15	0	0	0	0	0	0	0	0	0	0	0		
Count Total	8	22	23	55	108	1	3	0	0	4	1	2	3	2	8			
Peak Hour	3	13	9	27	52	0	2	0	0	2	0	1	1	2	4			

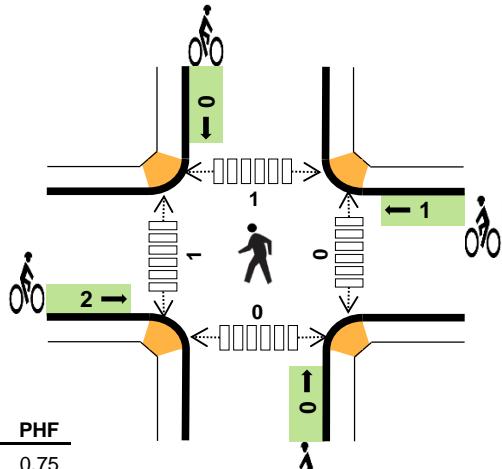
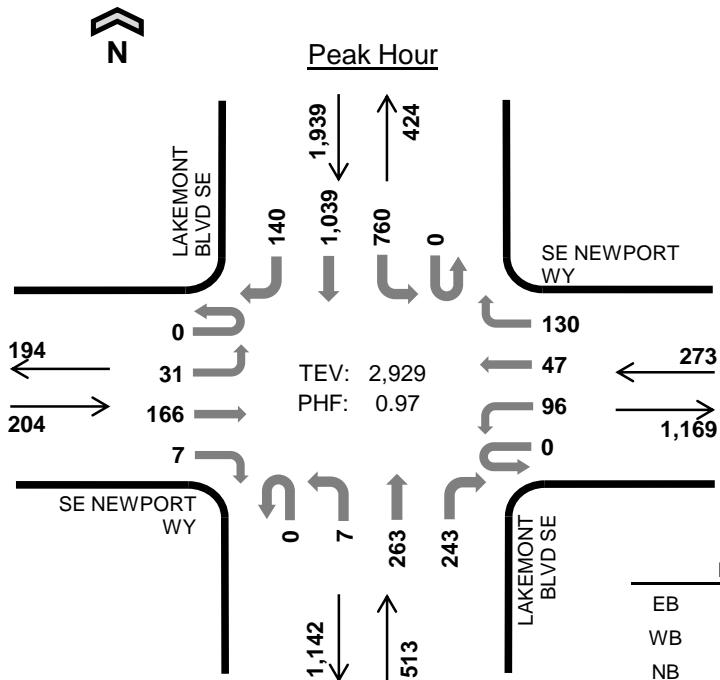
LAKEMONT BLVD SE SE NEWPORT WY



Date: Tue, Nov 13, 2018

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:15 PM to 5:15 PM



Two-Hour Count Summaries

Interval Start	SE NEWPORT WY				SE NEWPORT WY				LAKEMONT BLVD SE				LAKEMONT BLVD SE				15-min Total	Rolling One Hour	
	Eastbound		Westbound		Northbound		Southbound												
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	10	37	5	0	19	13	38	0	2	52	70	0	195	212	11	664	0	
4:15 PM	0	4	23	3	0	21	5	32	0	7	64	61	0	190	281	33	724	0	
4:30 PM	0	10	37	1	0	29	17	48	0	0	68	62	0	184	262	38	756	0	
4:45 PM	0	8	48	2	0	28	17	26	0	0	55	60	0	178	255	36	713	2,857	
5:00 PM	0	9	58	1	0	18	8	24	0	0	76	60	0	208	241	33	736	2,929	
5:15 PM	0	11	66	1	0	21	6	16	0	3	59	56	0	171	256	33	699	2,904	
5:30 PM	0	10	76	5	0	28	17	35	0	3	39	71	0	216	242	39	781	2,929	
5:45 PM	0	13	53	6	0	20	14	23	0	0	41	63	0	170	231	18	652	2,868	
Count Total	0	75	398	24	0	184	97	242	0	15	454	503	0	1,512	1,980	241	5,725	0	
Peak Hour	0	31	166	7	0	96	47	130	0	7	263	243	0	760	1,039	140	2,929	0	

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	5	0	6	4	15	0	1	0	0	1	0	0	1	0	1
4:15 PM	1	0	6	2	9	1	0	0	0	1	0	0	0	0	0
4:30 PM	1	0	3	7	11	0	0	0	0	0	0	1	1	0	2
4:45 PM	2	1	4	6	13	1	0	0	0	1	0	0	0	0	0
5:00 PM	2	0	1	1	4	0	1	0	0	1	0	0	0	0	0
5:15 PM	0	2	1	3	6	0	0	1	0	1	0	1	0	0	1
5:30 PM	1	0	2	1	4	0	0	0	0	0	1	0	0	1	2
5:45 PM	1	0	3	1	5	1	0	0	0	1	3	0	2	1	6
Count Total	13	3	26	25	67	3	2	1	0	6	4	2	4	2	12
Peak Hour	6	1	14	16	37	2	1	0	0	3	0	1	1	0	2

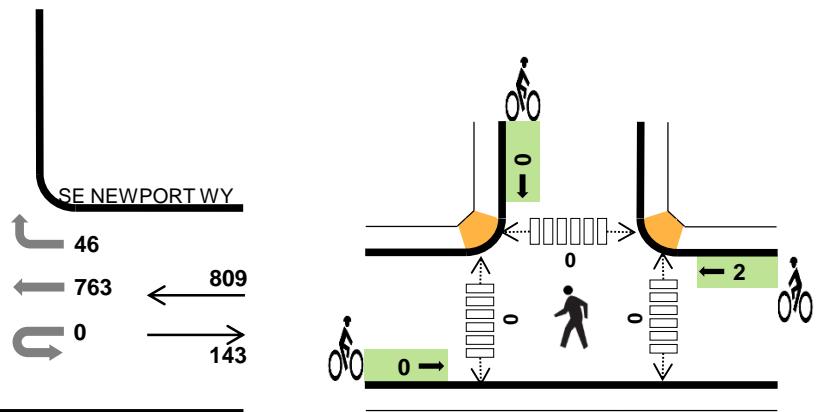
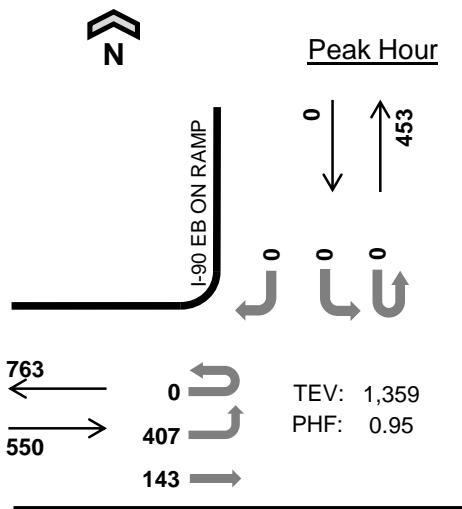
I-90 EB ON RAMP SE NEWPORT WY



Date: Tue, Nov 13, 2018

Count Period: 7:00 AM to 9:00 AM

Peak Hour: 7:45 AM to 8:45 AM



HV %:	PHF
EB	2.0%
WB	1.5%
NB	-
SB	-
TOTAL	1.7% 0.95

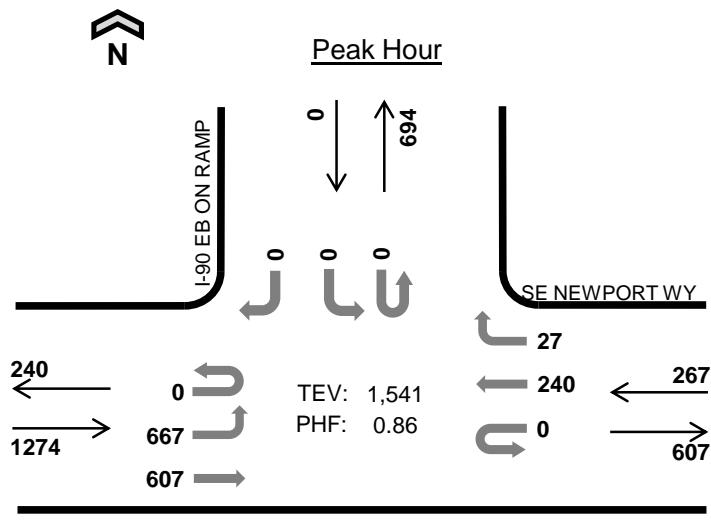
Two-Hour Count Summaries

Interval Start	SE NEWPORT WY				SE NEWPORT WY				0				I-90 EB ON RAMP				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
7:00 AM	0	114	22	0	0	0	103	7	0	0	0	0	0	0	0	0	246	0		
7:15 AM	0	126	23	0	0	0	148	5	0	0	0	0	0	0	0	0	302	0		
7:30 AM	0	128	30	0	0	0	161	3	0	0	0	0	0	0	0	0	322	0		
7:45 AM	0	118	31	0	0	0	191	19	0	0	0	0	0	0	0	0	359	1,229		
8:00 AM	0	71	35	0	0	0	220	7	0	0	0	0	0	0	0	0	333	1,316		
8:15 AM	0	108	44	0	0	0	183	8	0	0	0	0	0	0	0	0	343	1,357		
8:30 AM	0	110	33	0	0	0	169	12	0	0	0	0	0	0	0	0	324	1,359		
8:45 AM	0	106	30	0	0	0	153	9	0	0	0	0	0	0	0	0	298	1,298		
Count Total	0	881	248	0	0	0	1,328	70	0	0	0	0	0	0	0	0	2,527	0		
Peak Hour	0	407	143	0	0	0	763	46	0	0	0	0	0	0	0	0	1,359	0		

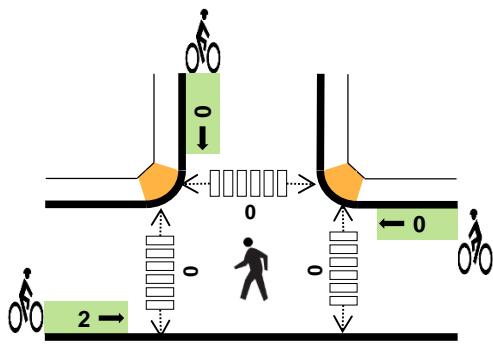
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals				Bicycles				Pedestrians (Crossing Leg)									
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South				
7:00 AM	6	1	0	0	7	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	7	2	0	0	9	0	1	0	0	1	0	0	1	0	0	1	0	1
7:30 AM	5	3	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	1	2	0	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	2	2	0	0	4	0	1	0	0	1	0							
8:15 AM	5	3	0	0	8	0	1	0	0	1	0	0	0	1	0	1	1	1
8:30 AM	3	5	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	5	3	0	0	8	0	0	0	0	0	0	0	0	0	0	0	0	0
Count Total	34	21	0	0	55	0	3	0	0	3	0	0	1	1	1	2		
Peak Hr	11	12	0	0	23	0	2	0	0	2	0	0	0	1	1	1		

I-90 EB ON RAMP SE NEWPORT WY



Date: Tue, Nov 13, 2018
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:45 PM to 5:45 PM



HV %:	PHF
EB	0.9% 0.88
WB	1.1% 0.78
NB	- -
SB	- -
TOTAL	1.0% 0.86

Two-Hour Count Summaries

Interval Start	SE NEWPORT WY				SE NEWPORT WY				0				I-90 EB ON RAMP				15-min Total	Rolling One Hour	
	Eastbound				Westbound				Northbound				Southbound						
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT			
4:00 PM	0	171	132	0	0	0	65	8	0	0	0	0	0	0	0	0	376	0	
4:15 PM	0	152	130	0	0	0	59	4	0	0	0	0	0	0	0	0	345	0	
4:30 PM	0	144	132	0	0	0	99	7	0	0	0	0	0	0	0	0	382	0	
4:45 PM	0	151	142	0	0	0	64	8	0	0	0	0	0	0	0	0	365	1,468	
5:00 PM	0	163	157	0	0	0	53	4	0	0	0	0	0	0	0	0	377	1,469	
5:15 PM	0	150	151	0	0	0	45	7	0	0	0	0	0	0	0	0	353	1,477	
5:30 PM	0	203	157	0	0	0	78	8	0	0	0	0	0	0	0	0	446	1,541	
5:45 PM	0	148	142	0	0	0	58	2	0	0	0	0	0	0	0	0	350	1,526	
Count Total	0	1,282	1,143	0	0	0	521	48	0	0	0	0	0	0	0	0	2,994	0	
Peak Hour	0	667	607	0	0	0	240	27	0	0	0	0	0	0	0	0	1,541	0	

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	8	1	0	0	9	0	1	0	0	1	0	0	0	0	0
4:15 PM	6	1	0	0	7	0	0	0	0	0	0	0	0	0	0
4:30 PM	6	2	0	0	8	1	0	0	0	1	0	0	1	0	1
4:45 PM	5	1	0	0	6	1	0	0	0	1	0	0	0	0	0
5:00 PM	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0
5:15 PM	2	2	0	0	4	1	0	0	0	1	0	0	0	0	0
5:30 PM	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	1	0	0	0	1	0	0	1	0	1
Count Total	32	7	0	0	39	4	1	0	0	5	0	0	2	0	2
Peak Hr	12	3	0	0	15	2	0	0	0	2	0	0	0	0	0

APPENDIX B: COLLISION DATA

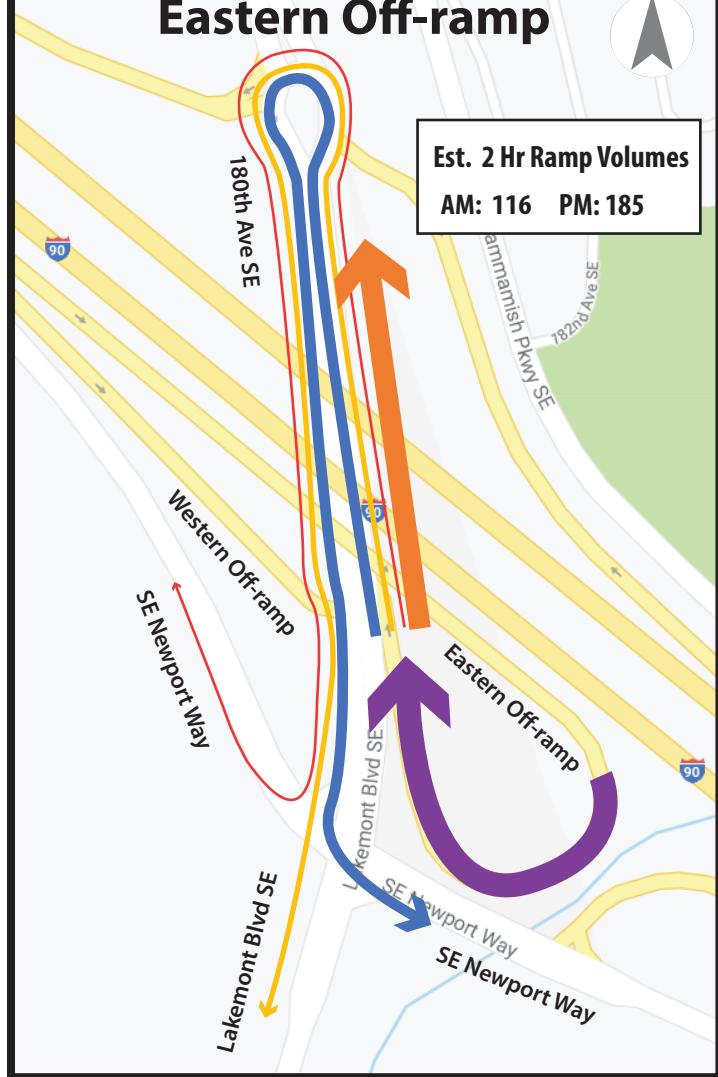
Date	Year	ColTypeDesc	PossInjuries	NDInjuries	DInjuries	Fatalities	Injury	RoadSurfaceDesc	LightCondDesc	VehDirLong	VehMovement	StreetName	CrossStreetName	Contrib1CircumDesc	Contrib2CircumDesc	Contrib3CircumDesc
1/25/2018 10:40	2018	Right Angle	0	0	0	0	PDO	wet	Daylight	Eastbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
1/25/2018 10:40	2018	Right Angle	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Unknown driver distraction		
8/31/2017 15:47	2017	Approach Turn	2	0	0	0	Possible Injury	dry	Daylight	Southbound	LT	SE NEWPORT WY	LAKEMONT BD SE	None		
8/31/2017 15:47	2017	Approach Turn	2	0	0	0	Possible Injury	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Disregard Stop and Go Signal	Did Not Grant R/W to Vehicle	
8/31/2017 15:47	2017	Approach Turn	2	0	0	0	Possible Injury	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
8/31/2017 15:47	2017	Approach Turn	2	0	0	0	Possible Injury	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
6/25/2017 23:05	2017	Other	1	0	0	0	Possible Injury	dry	Dark - Street Lights On	Northbound	UT	SE NEWPORT WY	LAKEMONT BD SE	Improper U Turn		
6/25/2017 23:05	2017	Other	1	0	0	0	Possible Injury	dry	Dark - Street Lights On	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
6/19/2017 17:55	2017	Rear End	1	0	0	0	Possible Injury	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Inattention		
6/19/2017 17:55	2017	Rear End	1	0	0	0	Possible Injury	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
4/11/2017 15:52	2017	Sideswipe/Lane Change	0	0	0	0	PDO	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Did Not Grant R/W to Vehicle		
4/11/2017 15:52	2017	Sideswipe/Lane Change	0	0	0	0	PDO	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
10/19/2016 22:00	2016	Right Angle	0	0	0	0	PDO	wet	Dark - Street Lights On	Westbound	T	SE NEWPORT WY	LAKEMONT BD SE	Under Influence of Drugs		
10/19/2016 22:00	2016	Right Angle	0	0	0	0	PDO	wet	Dark - Street Lights On	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
9/7/2016 18:40	2016	Rear End	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Exceeding Reasonable Safe Speed		
9/7/2016 18:40	2016	Rear End	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
8/20/2016 15:53	2016	Rear End	2	0	0	0	Possible Injury	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	Other * (List in Narrative)		
8/20/2016 15:53	2016	Rear End	2	0	0	0	Possible Injury	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
5/3/2016 7:25	2016	Rear End	0	0	0	0	PDO	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	Inattention	Did Not Grant R/W to Vehicle	
5/3/2016 7:25	2016	Rear End	0	0	0	0	PDO	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	None		
4/21/2016 11:32	2016	Rear End	0	1	0	0	Non-Disabling Injury	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Inattention	Following Too Closely	
4/21/2016 11:32	2016	Rear End	0	1	0	0	Non-Disabling Injury	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
12/22/2015 10:17	2015	Rear End	0	0	0	0	PDO	wet	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	Exceeding Reasonable Safe Speed		
12/22/2015 10:17	2015	Rear End	0	0	0	0	PDO	wet	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
10/1/2015 14:32	2015	Sideswipe/Lane Change	0	0	0	0	PDO	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	Did Not Grant R/W to Vehicle		
10/1/2015 14:32	2015	Sideswipe/Lane Change	0	0	0	0	PDO	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
5/29/2015 16:38	2015	Rear End	0	0	0	0	PDO	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	Inattention		
5/29/2015 16:38	2015	Rear End	0	0	0	0	PDO	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	None		
10/16/2014 8:13	2014	Rear End	0	0	0	0	PDO	dry	Daylight	Westbound	T	SE NEWPORT WY	LAKEMONT BD SE	Inattention		
10/16/2014 8:13	2014	Rear End	0	0	0	0	PDO	dry	Daylight	Westbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
10/15/2014 11:25	2014	Sideswipe/Lane Change	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Inattention		
10/15/2014 11:25	2014	Sideswipe/Lane Change	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
8/5/2014 7:13	2014	Parked Vehicle/Fixed Object	0	6	0	0	Non-Disabling Injury	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	Inattention		
6/23/2014 17:01	2014	Rear End	0	0	0	0	PDO	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	Exceeding Reasonable Safe Speed		
6/23/2014 17:01	2014	Rear End	0	0	0	0	PDO	dry	Daylight	Northbound	RT	SE NEWPORT WY	LAKEMONT BD SE	None		
5/29/2014 19:31	2014	Parked Vehicle/Fixed Object	0	1	0	0	Non-Disabling Injury	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Under Influence of Alcohol		
10/27/2013 14:19	2013	Rear End	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	Inattention	Exceeding Reasonable Safe Speed	Unknown driver distraction
10/27/2013 14:19	2013	Rear End	0	0	0	0	PDO	wet	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
6/8/2013 17:40	2013	Right Angle	0	0	0	0	PDO	dry	Daylight	Westbound	T	SE NEWPORT WY	LAKEMONT BD SE	Disregard Stop and Go Signal		
6/8/2013 17:40	2013	Right Angle	0	0	0	0	PDO	dry	Daylight	Northbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
6/8/2013 17:40	2013	Right Angle	0	0	0	0	PDO	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		
2/8/2013 12:30	2013	Rear End	1	0	0	0	Possible Injury	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	Exceeding Reasonable Safe Speed		
2/8/2013 12:30	2013	Rear End	1	0	0	0	Possible Injury	dry	Daylight	Southbound	T	SE NEWPORT WY	LAKEMONT BD SE	None		

APPENDIX C: ORIGIN-DESTINATION STUDY

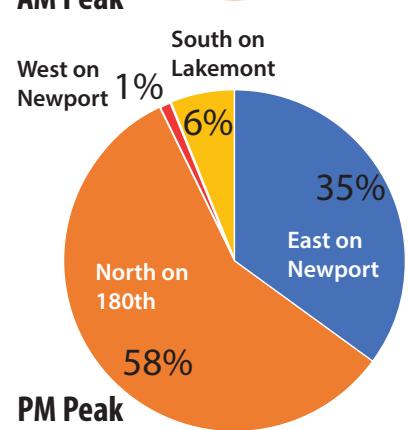
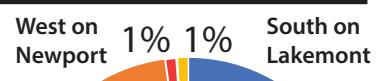
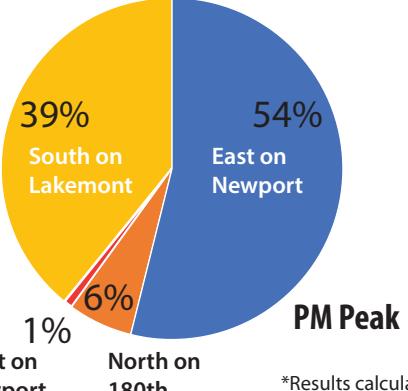
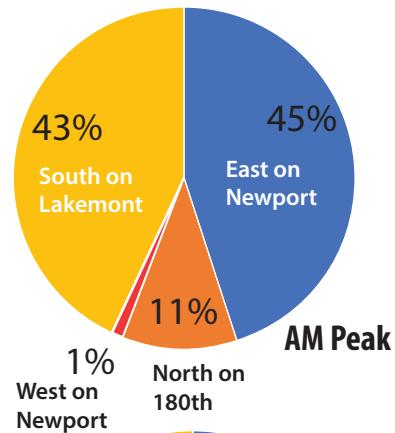
Western Off-ramp



Eastern Off-ramp



Bellevue Exit 13 Origin-Destination Analysis



*Results calculated using Streetlight data from 2017-2018 calibrated with WSDOT 2017 AADT counts at each ramp.

APPENDIX D: TRAFFIC ANALYSIS

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	30	42	7	67	177	519	18	658	295	222	354	65
Future Volume (vph)	30	42	7	67	177	519	18	658	295	222	354	65
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	140		140		40	275	0	285
Storage Lanes	1			0	1		1		1	1	1	1
Taper Length (ft)	75				75			100			150	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.91	0.91	1.00
Ped Bike Factor	1.00	1.00			1.00		0.98	1.00				0.98
Fr _t		0.979				0.850		0.954				0.850
Flt Protected	0.950				0.950			0.950			0.950	0.995
Satd. Flow (prot)	1736	1785	0	1770	1863	1583	1787	3410	0	1579	3308	1553
Flt Permitted	0.950			0.950			0.950			0.950	0.995	
Satd. Flow (perm)	1734	1785	0	1764	1863	1559	1783	3410	0	1579	3308	1515
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		8				233		72				125
Link Speed (mph)		40			30			40			30	
Link Distance (ft)		989			412			695			444	
Travel Time (s)		16.9			9.4			23.7			9.1	
Confl. Peds. (#/hr)	1		2	2		1	1					1
Confl. Bikes (#/hr)				2								
Peak Hour Factor	0.76	0.76	0.76	0.88	0.88	0.88	0.91	0.91	0.91	0.92	0.92	0.92
Heavy Vehicles (%)	4%	4%	4%	2%	2%	2%	1%	1%	1%	4%	4%	4%
Adj. Flow (vph)	39	55	9	76	201	590	20	723	324	241	385	71
Shared Lane Traffic (%)										16%		
Lane Group Flow (vph)	39	64	0	76	201	590	20	1047	0	202	424	71
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		10			10			10			10	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	3	2		2	4	2	2	2		2	3	2
Detector Template												
Leading Detector (ft)	156	100		100	236	100	100	100		100	176	100
Trailing Detector (ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Position(ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)	94	94		94	94	94	94	94		94	94	94
Detector 2 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 2 Channel												

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 3 Position(ft)	150				150						170	
Detector 3 Size(ft)	6				6						6	
Detector 3 Type	Cl+Ex				Cl+Ex						Cl+Ex	
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0						0.0	
Detector 4 Position(ft)					230							
Detector 4 Size(ft)					6							
Detector 4 Type					Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)					0.0							
Turn Type	Prot	NA		Prot	NA	pm+ov	Split	NA		Split	NA	Perm
Protected Phases	3	8		7	4	2	1	1		2	2	
Permitted Phases					4						2	
Detector Phase	3	8		7	4	2	1	1		2	2	2
Switch Phase												
Minimum Initial (s)	5.0	12.0		5.0	12.0	5.0	7.0	7.0		5.0	5.0	5.0
Minimum Split (s)	10.0	23.5		10.0	24.5	17.5	25.5	25.5		17.5	17.5	17.5
Total Split (s)	15.0	25.0		15.0	25.0	28.0	32.0	32.0		28.0	28.0	28.0
Total Split (%)	15.0%	25.0%		15.0%	25.0%	28.0%	32.0%	32.0%		28.0%	28.0%	28.0%
Maximum Green (s)	10.5	20.5		10.5	20.5	22.5	26.5	26.5		22.5	22.5	22.5
Yellow Time (s)	3.5	3.5		3.5	3.5	4.5	4.5	4.5		4.5	4.5	4.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	2.5	2.5		2.5	2.5	3.5	3.5	3.5		3.5	3.5	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Minimum Gap (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Time Before Reduce (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Time To Reduce (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Recall Mode	None	None		None	None	None	None	None		None	None	None
Walk Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	5.0
Flash Dont Walk (s)		14.0			15.0	7.0	15.0	15.0		7.0	7.0	7.0
Pedestrian Calls (#/hr)		1			1	1	0	0		1	1	1
Act Effct Green (s)	8.6	15.9		9.9	19.2	39.9	29.0	29.0		21.8	21.8	21.8
Actuated g/C Ratio	0.10	0.18		0.11	0.22	0.46	0.34	0.34		0.25	0.25	0.25
v/c Ratio	0.23	0.19		0.37	0.49	0.69	0.03	0.88		0.51	0.51	0.15
Control Delay	42.1	30.5		43.6	35.9	15.0	23.8	37.3		34.2	31.0	1.6
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	42.1	30.5		43.6	35.9	15.0	23.8	37.3		34.2	31.0	1.6
LOS	D	C		D	D	B	C	D		C	C	A
Approach Delay		34.9			22.4			37.0			28.9	
Approach LOS		C			C			D			C	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 86.4

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 30.2

Intersection LOS: C

Intersection Capacity Utilization 74.0%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 228: Lakemont Blvd & Newport Way



Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	31	181	7	96	47	130	7	263	265	828	1039	140
Future Volume (vph)	31	181	7	96	47	130	7	263	265	828	1039	140
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	140		140	40		275	0	285
Storage Lanes	1			0	1		1	1		1	1	1
Taper Length (ft)	75				75			100			150	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.91	0.91	1.00
Ped Bike Factor	1.00	1.00				0.98	1.00					0.97
Fr _t		0.995				0.850		0.925				0.850
Flt Protected	0.950				0.950			0.950			0.950	0.991
Satd. Flow (prot)	1752	1834	0	1805	1900	1615	1752	3242	0	1626	3393	1599
Flt Permitted	0.950			0.950			0.950			0.950	0.991	
Satd. Flow (perm)	1749	1834	0	1805	1900	1590	1751	3242	0	1626	3393	1559
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				178		175				115
Link Speed (mph)		40			30			40			30	
Link Distance (ft)		989			412			695			444	
Travel Time (s)		16.9			9.4			23.7			9.1	
Confl. Peds. (#/hr)	1					1	1					1
Confl. Bikes (#/hr)		2				1						
Peak Hour Factor	0.75	0.75	0.75	0.73	0.73	0.73	0.94	0.94	0.94	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	0%	0%	0%	3%	3%	3%	1%	1%	1%
Adj. Flow (vph)	41	241	9	132	64	178	7	280	282	863	1082	146
Shared Lane Traffic (%)										27%		
Lane Group Flow (vph)	41	250	0	132	64	178	7	562	0	630	1315	146
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		10			10			10			10	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	3	2		2	4	2	2	2		2	3	2
Detector Template												
Leading Detector (ft)	156	100		100	236	100	100	100		100	176	100
Trailing Detector (ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Position(ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)	94	94		94	94	94	94	94		94	94	94
Detector 2 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 2 Channel												

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 3 Position(ft)	150				150						170	
Detector 3 Size(ft)	6				6						6	
Detector 3 Type	Cl+Ex				Cl+Ex						Cl+Ex	
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0						0.0	
Detector 4 Position(ft)					230							
Detector 4 Size(ft)					6							
Detector 4 Type					Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)					0.0							
Turn Type	Prot	NA		Prot	NA	pm+ov	Split	NA		Split	NA	Perm
Protected Phases	3	8		7	4	2	1	1		2	2	
Permitted Phases					4						2	
Detector Phase	3	8		7	4	2	1	1		2	2	2
Switch Phase												
Minimum Initial (s)	5.0	12.0		5.0	12.0	5.0	7.0	7.0		5.0	5.0	5.0
Minimum Split (s)	10.0	23.5		10.0	24.5	17.5	25.5	25.5		17.5	17.5	17.5
Total Split (s)	18.0	27.0		18.0	27.0	55.0	30.0	30.0		55.0	55.0	55.0
Total Split (%)	13.8%	20.8%		13.8%	20.8%	42.3%	23.1%	23.1%		42.3%	42.3%	42.3%
Maximum Green (s)	13.5	22.5		13.5	22.5	49.5	24.5	24.5		49.5	49.5	49.5
Yellow Time (s)	3.5	3.5		3.5	3.5	4.5	4.5	4.5		4.5	4.5	4.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	2.5	2.5		2.5	2.5	3.5	3.5	3.5		3.5	3.5	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Minimum Gap (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Time Before Reduce (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Time To Reduce (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Recall Mode	None	None		None	None	None	None	None		None	None	None
Walk Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	5.0
Flash Dont Walk (s)		14.0			15.0	7.0	15.0	15.0		7.0	7.0	7.0
Pedestrian Calls (#/hr)	0			1	1	0	0			1	1	1
Act Effct Green (s)	9.4	21.0		13.6	27.5	78.4	21.0	21.0		51.9	51.9	51.9
Actuated g/C Ratio	0.08	0.18		0.11	0.23	0.66	0.18	0.18		0.43	0.43	0.43
v/c Ratio	0.30	0.77		0.65	0.15	0.16	0.02	0.79		0.89	0.89	0.20
Control Delay	60.5	64.6		67.6	40.7	1.7	42.0	41.2		49.9	41.9	7.8
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	60.5	64.6		67.6	40.7	1.7	42.0	41.2		49.9	41.9	7.8
LOS	E	E		E	D	A	D	D		D	D	A
Approach Delay		64.0			31.6			41.2			41.9	
Approach LOS		E			C			D			D	

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 119.6

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.89

Intersection Signal Delay: 42.6

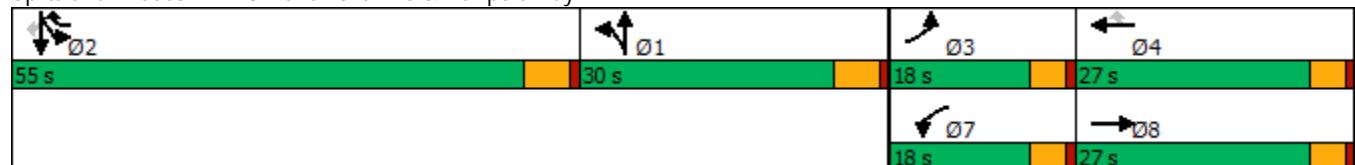
Intersection LOS: D

Intersection Capacity Utilization 79.6%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 228: Lakemont Blvd & Newport Way



Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	30	60	10	40	220	480	20	600	330	370	340	120
Future Volume (vph)	30	60	10	40	220	480	20	600	330	370	340	120
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	140		140		40		275	0
Storage Lanes	1			0	1		1		1		1	1
Taper Length (ft)	75				75			100			150	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.91	0.91	1.00
Ped Bike Factor	1.00	1.00			1.00		0.98	1.00				0.98
Fr _t		0.979				0.850		0.947				0.850
Flt Protected	0.950				0.950			0.950			0.950	0.986
Satd. Flow (prot)	1736	1785	0	1770	1863	1583	1787	3385	0	1579	3278	1553
Flt Permitted	0.950			0.950			0.950			0.950	0.986	
Satd. Flow (perm)	1734	1785	0	1765	1863	1559	1784	3385	0	1579	3278	1515
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		8				243		104				130
Link Speed (mph)		40			30			40			30	
Link Distance (ft)		989			412			695			444	
Travel Time (s)		16.9			9.4			23.7			9.1	
Confl. Peds. (#/hr)	1		2	2		1	1					1
Confl. Bikes (#/hr)				2								
Peak Hour Factor	0.76	0.76	0.76	0.88	0.88	0.88	0.91	0.91	0.91	0.92	0.92	0.92
Heavy Vehicles (%)	4%	4%	4%	2%	2%	2%	1%	1%	1%	4%	4%	4%
Adj. Flow (vph)	39	79	13	45	250	545	22	659	363	402	370	130
Shared Lane Traffic (%)										37%		
Lane Group Flow (vph)	39	92	0	45	250	545	22	1022	0	253	519	130
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		10			10			10			10	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	3	2		2	4	2	2	2		2	3	2
Detector Template												
Leading Detector (ft)	156	100		100	236	100	100	100		100	176	100
Trailing Detector (ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Position(ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)	94	94		94	94	94	94	94		94	94	94
Detector 2 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 2 Channel												

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 3 Position(ft)	150				150						170	
Detector 3 Size(ft)	6				6						6	
Detector 3 Type	Cl+Ex				Cl+Ex						Cl+Ex	
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0						0.0	
Detector 4 Position(ft)					230							
Detector 4 Size(ft)					6							
Detector 4 Type					Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)					0.0							
Turn Type	Prot	NA		Prot	NA	pm+ov	Split	NA		Split	NA	Perm
Protected Phases	3	8		7	4	2	1	1		2	2	
Permitted Phases					4						2	
Detector Phase	3	8		7	4	2	1	1		2	2	2
Switch Phase												
Minimum Initial (s)	5.0	12.0		5.0	12.0	5.0	7.0	7.0		5.0	5.0	5.0
Minimum Split (s)	10.0	23.5		10.0	24.5	17.5	25.5	25.5		17.5	17.5	17.5
Total Split (s)	15.0	25.0		15.0	25.0	28.0	32.0	32.0		28.0	28.0	28.0
Total Split (%)	15.0%	25.0%		15.0%	25.0%	28.0%	32.0%	32.0%		28.0%	28.0%	28.0%
Maximum Green (s)	10.5	20.5		10.5	20.5	22.5	26.5	26.5		22.5	22.5	22.5
Yellow Time (s)	3.5	3.5		3.5	3.5	4.5	4.5	4.5		4.5	4.5	4.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	2.5	2.5		2.5	2.5	3.5	3.5	3.5		3.5	3.5	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Minimum Gap (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Time Before Reduce (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Time To Reduce (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Recall Mode	None	None		None	None	None	None	None		None	None	None
Walk Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	5.0
Flash Dont Walk (s)		14.0			15.0	7.0	15.0	15.0		7.0	7.0	7.0
Pedestrian Calls (#/hr)		1			1	1	0	0		1	1	1
Act Effct Green (s)	8.7	17.8		8.9	18.0	40.4	28.7	28.7		23.4	23.4	23.4
Actuated g/C Ratio	0.10	0.21		0.10	0.21	0.47	0.33	0.33		0.27	0.27	0.27
v/c Ratio	0.23	0.25		0.25	0.64	0.63	0.04	0.86		0.59	0.59	0.26
Control Delay	42.5	30.2		42.5	41.1	12.3	24.4	35.0		36.2	32.0	7.0
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	42.5	30.2		42.5	41.1	12.3	24.4	35.0		36.2	32.0	7.0
LOS	D	C		D	D	B	C	C		D	C	A
Approach Delay		33.9			22.5			34.7			29.5	
Approach LOS		C			C			C			C	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 86.4

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.86

Intersection Signal Delay: 29.6

Intersection LOS: C

Intersection Capacity Utilization 71.2%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 228: Lakemont Blvd & Newport Way



Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑		↑	↑↑	↑
Traffic Volume (vph)	30	170	0	80	50	140	10	230	270	1130	970	160
Future Volume (vph)	30	170	0	80	50	140	10	230	270	1130	970	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	140		140	40		275	0	285
Storage Lanes	1			0	1		1	1		1	1	1
Taper Length (ft)	75			75			100			150		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.91	0.91	1.00
Ped Bike Factor	1.00					0.98	1.00					0.97
Fr _t						0.850		0.919				0.850
Flt Protected	0.950				0.950			0.950			0.950	0.985
Satd. Flow (prot)	1752	1845	0	1805	1900	1615	1752	3221	0	1626	3372	1599
Flt Permitted	0.950			0.950			0.950			0.950	0.985	
Satd. Flow (perm)	1749	1845	0	1805	1900	1590	1751	3221	0	1626	3372	1559
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						192		204				118
Link Speed (mph)		40			30			40			30	
Link Distance (ft)		989			412			695			444	
Travel Time (s)		16.9			9.4			23.7			9.1	
Confl. Peds. (#/hr)	1					1	1					1
Confl. Bikes (#/hr)		2				1						
Peak Hour Factor	0.75	0.75	0.75	0.73	0.73	0.73	0.94	0.94	0.94	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	0%	0%	0%	3%	3%	3%	1%	1%	1%
Adj. Flow (vph)	40	227	0	110	68	192	11	245	287	1177	1010	167
Shared Lane Traffic (%)										39%		
Lane Group Flow (vph)	40	227	0	110	68	192	11	532	0	718	1469	167
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		10			10			10			10	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	3	2		2	4	2	2	2		2	3	2
Detector Template												
Leading Detector (ft)	156	100		100	236	100	100	100		100	176	100
Trailing Detector (ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Position(ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)	94	94		94	94	94	94	94		94	94	94
Detector 2 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 2 Channel												

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 3 Position(ft)	150				150						170	
Detector 3 Size(ft)	6				6						6	
Detector 3 Type	Cl+Ex				Cl+Ex						Cl+Ex	
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0						0.0	
Detector 4 Position(ft)					230							
Detector 4 Size(ft)					6							
Detector 4 Type					Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)					0.0							
Turn Type	Prot	NA		Prot	NA	pm+ov	Split	NA		Split	NA	Perm
Protected Phases	3	8		7	4	2	1	1		2	2	
Permitted Phases					4						2	
Detector Phase	3	8		7	4	2	1	1		2	2	2
Switch Phase												
Minimum Initial (s)	5.0	12.0		5.0	12.0	5.0	7.0	7.0		5.0	5.0	5.0
Minimum Split (s)	10.0	23.5		10.0	24.5	17.5	25.5	25.5		17.5	17.5	17.5
Total Split (s)	18.0	27.0		18.0	27.0	55.0	30.0	30.0		55.0	55.0	55.0
Total Split (%)	13.8%	20.8%		13.8%	20.8%	42.3%	23.1%	23.1%		42.3%	42.3%	42.3%
Maximum Green (s)	13.5	22.5		13.5	22.5	49.5	24.5	24.5		49.5	49.5	49.5
Yellow Time (s)	3.5	3.5		3.5	3.5	4.5	4.5	4.5		4.5	4.5	4.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	2.5	2.5		2.5	2.5	3.5	3.5	3.5		3.5	3.5	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Minimum Gap (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Time Before Reduce (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Time To Reduce (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Recall Mode	None	None		None	None	None	None	None		None	None	None
Walk Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	5.0
Flash Dont Walk (s)		14.0			15.0	7.0	15.0	15.0		7.0	7.0	7.0
Pedestrian Calls (#/hr)	0			1	1	0	0			1	1	1
Act Effct Green (s)	9.2	20.0		12.7	27.9	78.9	18.9	18.9		52.0	52.0	52.0
Actuated g/C Ratio	0.08	0.17		0.11	0.24	0.68	0.16	0.16		0.45	0.45	0.45
v/c Ratio	0.29	0.71		0.56	0.15	0.17	0.04	0.76		0.98	0.97	0.22
Control Delay	58.8	59.6		62.1	39.2	1.6	42.3	36.3		63.1	49.7	8.7
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	58.8	59.6		62.1	39.2	1.6	42.3	36.3		63.1	49.7	8.7
LOS	E	E		E	D	A	D	D		E	D	A
Approach Delay		59.5			26.5			36.4			50.9	
Approach LOS		E			C			D			D	

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 115.7

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Natural Cycle: 110

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 46.7

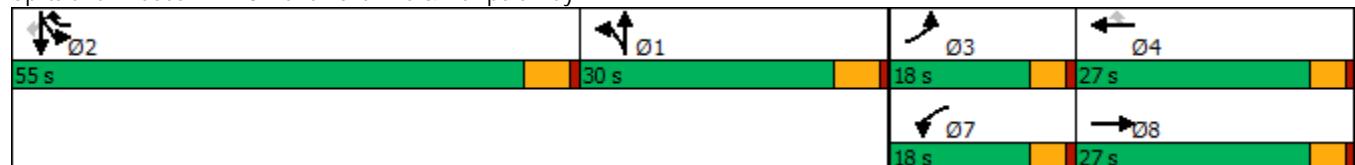
Intersection LOS: D

Intersection Capacity Utilization 82.6%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 228: Lakemont Blvd & Newport Way



Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↑	↑	↑	↑	↑↑		↑↑	↑	↑
Traffic Volume (vph)	30	60	10	40	220	480	20	600	330	370	340	120
Future Volume (vph)	30	60	10	40	220	480	20	600	330	370	340	120
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			140		140	40		275	0		285
Storage Lanes	1			1		1	1		1	2		1
Taper Length (ft)	75			75			100			150		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	1.00	1.00
Ped Bike Factor	1.00	1.00		1.00		0.99	1.00					0.98
Fr _t		0.979				0.850		0.947				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1736	1785	0	1770	1863	1583	1787	3385	0	3367	1827	1553
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1734	1785	0	1766	1863	1561	1784	3385	0	3367	1827	1517
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		10				194		135				157
Link Speed (mph)		40			30			40			30	
Link Distance (ft)		989			412			695			444	
Travel Time (s)		16.9			9.4			23.7			9.1	
Confl. Peds. (#/hr)	1		2	2		1	1					1
Confl. Bikes (#/hr)				2								
Peak Hour Factor	0.76	0.76	0.76	0.88	0.88	0.88	0.91	0.91	0.91	0.92	0.92	0.92
Heavy Vehicles (%)	4%	4%	4%	2%	2%	2%	1%	1%	1%	4%	4%	4%
Adj. Flow (vph)	39	79	13	45	250	545	22	659	363	402	370	130
Shared Lane Traffic (%)												
Lane Group Flow (vph)	39	92	0	45	250	545	22	1022	0	402	370	130
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		10			10			10			10	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	3	2		2	4	2	2	2		2	3	2
Detector Template												
Leading Detector (ft)	156	100		100	236	100	100	100		100	176	100
Trailing Detector (ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Position(ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)	94	94		94	94	94	94	94		94	94	94
Detector 2 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 2 Channel												

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 3 Position(ft)	150				150						170	
Detector 3 Size(ft)	6				6						6	
Detector 3 Type	Cl+Ex				Cl+Ex						Cl+Ex	
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0						0.0	
Detector 4 Position(ft)					230							
Detector 4 Size(ft)					6							
Detector 4 Type					Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)					0.0							
Turn Type	Prot	NA		Prot	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases	3	8		7	4	5	1	6		5	2	
Permitted Phases						4					2	
Detector Phase	3	8		7	4	5	1	6		5	2	2
Switch Phase												
Minimum Initial (s)	5.0	12.0		5.0	12.0	5.0	7.0	7.0		5.0	5.0	5.0
Minimum Split (s)	9.5	23.5		9.5	24.5	17.5	25.5	25.5		17.5	17.5	17.5
Total Split (s)	10.0	24.0		10.0	24.0	18.0	24.0	28.0		18.0	22.0	22.0
Total Split (%)	12.5%	30.0%		12.5%	30.0%	22.5%	30.0%	35.0%		22.5%	27.5%	27.5%
Maximum Green (s)	5.5	19.5		5.5	19.5	12.5	18.5	22.5		12.5	16.5	16.5
Yellow Time (s)	3.5	3.5		3.5	3.5	4.5	4.5	4.5		4.5	4.5	4.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	2.5	2.5		2.5	2.5	3.5	3.5	3.5		3.5	3.5	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Minimum Gap (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Time Before Reduce (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Time To Reduce (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Recall Mode	None	None		None	None	None	None	None		None	None	None
Walk Time (s)		5.0			5.0			5.0			5.0	5.0
Flash Dont Walk (s)		14.0			15.0			15.0			7.0	7.0
Pedestrian Calls (#/hr)		1			1			0			1	1
Act Effct Green (s)	7.5	17.0		7.5	17.0	29.4	13.3	23.2		13.4	31.8	31.8
Actuated g/C Ratio	0.11	0.25		0.11	0.25	0.43	0.19	0.34		0.20	0.46	0.46
v/c Ratio	0.21	0.20		0.23	0.54	0.70	0.06	0.83		0.61	0.44	0.17
Control Delay	35.2	22.0		35.5	29.2	14.9	23.9	27.0		31.5	22.7	3.9
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	35.2	22.0		35.5	29.2	14.9	23.9	27.0		31.5	22.7	3.9
LOS	D	C		D	C	B	C	C		C	C	A
Approach Delay		25.9			20.3			27.0			23.9	
Approach LOS		C			C			C			C	

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 68.7

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 24.0

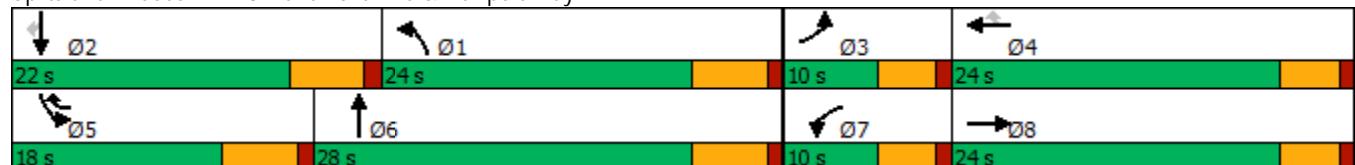
Intersection LOS: C

Intersection Capacity Utilization 71.2%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 228: Lakemont Blvd & Newport Way



Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	30	170	0	80	50	140	10	230	270	1130	970	160
Future Volume (vph)	30	170	0	80	50	140	10	230	270	1130	970	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	200			0	140		140	40		275	0	285
Storage Lanes	1			0	1		1	1		1	2	1
Taper Length (ft)	75			75			100			150		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	1.00	1.00
Ped Bike Factor	1.00					0.99	1.00					0.97
Fr _t						0.850		0.919				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1752	1845	0	1805	1900	1615	1752	3221	0	3467	1881	1599
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1749	1845	0	1805	1900	1592	1750	3221	0	3467	1881	1559
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)						192		151				126
Link Speed (mph)		40			30			40			30	
Link Distance (ft)		989			412			695			444	
Travel Time (s)		16.9			9.4			23.7			9.1	
Confl. Peds. (#/hr)	1					1	1					1
Confl. Bikes (#/hr)		2				1						
Peak Hour Factor	0.75	0.75	0.75	0.73	0.73	0.73	0.94	0.94	0.94	0.96	0.96	0.96
Heavy Vehicles (%)	3%	3%	3%	0%	0%	0%	3%	3%	3%	1%	1%	1%
Adj. Flow (vph)	40	227	0	110	68	192	11	245	287	1177	1010	167
Shared Lane Traffic (%)												
Lane Group Flow (vph)	40	227	0	110	68	192	11	532	0	1177	1010	167
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		10			10			10			10	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	3	2		2	4	2	2	2		2	3	2
Detector Template												
Leading Detector (ft)	156	100		100	236	100	100	100		100	176	100
Trailing Detector (ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Position(ft)	2	2		2	2	2	2	2		2	2	2
Detector 1 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)	94	94		94	94	94	94	94		94	94	94
Detector 2 Size(ft)	6	6		6	6	6	6	6		6	6	6
Detector 2 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 2 Channel												

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector 2 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 3 Position(ft)	150				150						170	
Detector 3 Size(ft)	6				6						6	
Detector 3 Type	Cl+Ex				Cl+Ex						Cl+Ex	
Detector 3 Channel												
Detector 3 Extend (s)	0.0				0.0						0.0	
Detector 4 Position(ft)					230							
Detector 4 Size(ft)					6							
Detector 4 Type					Cl+Ex							
Detector 4 Channel												
Detector 4 Extend (s)					0.0							
Turn Type	Prot	NA		Prot	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases	3	8		7	4	5	1	6		5	2	
Permitted Phases						4					2	
Detector Phase	3	8		7	4	5	1	6		5	2	2
Switch Phase												
Minimum Initial (s)	5.0	12.0		5.0	12.0	5.0	7.0	7.0		5.0	5.0	5.0
Minimum Split (s)	10.0	23.5		10.0	24.5	17.5	25.5	25.5		17.5	17.5	17.5
Total Split (s)	11.0	24.0		11.0	24.0	58.0	20.0	37.0		58.0	75.0	75.0
Total Split (%)	8.5%	18.5%		8.5%	18.5%	44.6%	15.4%	28.5%		44.6%	57.7%	57.7%
Maximum Green (s)	6.5	19.5		6.5	19.5	52.5	14.5	31.5		52.5	69.5	69.5
Yellow Time (s)	3.5	3.5		3.5	3.5	4.5	4.5	4.5		4.5	4.5	4.5
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0		-2.0	-2.0	-2.0	-2.0	-2.0		-2.0	-2.0	-2.0
Total Lost Time (s)	2.5	2.5		2.5	2.5	3.5	3.5	3.5		3.5	3.5	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Minimum Gap (s)	2.0	2.0		2.0	2.0	2.0	2.0	2.0		2.0	2.0	2.0
Time Before Reduce (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Time To Reduce (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Recall Mode	None	None		None	None	None	None	None		None	None	None
Walk Time (s)		5.0			5.0			5.0			5.0	5.0
Flash Dont Walk (s)		14.0			15.0			15.0			7.0	7.0
Pedestrian Calls (#/hr)	0			1			0			1	1	
Act Effct Green (s)	8.1	19.0		8.7	24.2	68.8	9.7	22.4		45.6	69.0	69.0
Actuated g/C Ratio	0.08	0.18		0.08	0.22	0.64	0.09	0.21		0.42	0.64	0.64
v/c Ratio	0.31	0.70		0.76	0.16	0.18	0.07	0.68		0.80	0.84	0.16
Control Delay	58.7	56.3		84.0	41.4	1.6	51.8	33.4		32.9	24.6	3.4
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	58.7	56.3		84.0	41.4	1.6	51.8	33.4		32.9	24.6	3.4
LOS	E	E		F	D	A	D	C		C	C	A
Approach Delay		56.6			33.4			33.8			27.2	
Approach LOS		E			C			C			C	

Intersection Summary

Area Type: Other

Cycle Length: 130

Actuated Cycle Length: 108

Lanes, Volumes, Timings
228: Lakemont Blvd & Newport Way

05/28/2019

Natural Cycle: 130

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 31.1

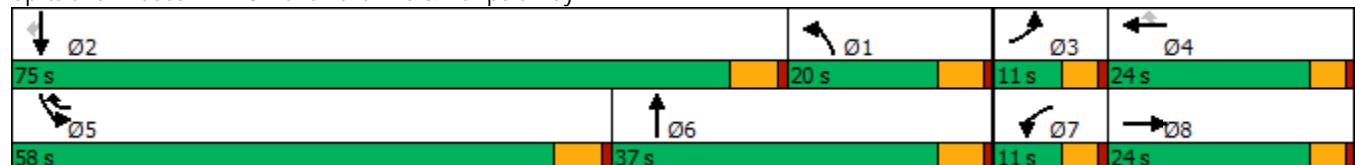
Intersection LOS: C

Intersection Capacity Utilization 84.7%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 228: Lakemont Blvd & Newport Way



MOVEMENT SUMMARY

Site: 10 [Newport and Lakemont AM]

Roundabout

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Queue Distance ft	Prop. Queued	Effective Stop Rate per veh	Average Speed mph
South: Lakemont Blvd											
3	L2	22	1.0	0.608	13.1	LOS A	4.8	122.1	0.71	0.73	35.6
8	T1	659	1.0	0.608	7.3	LOS A	4.8	122.1	0.71	0.73	31.8
18	R2	363	1.0	0.219	3.6	LOS A	0.0	0.0	0.00	0.44	37.3
Approach		1044	1.0	0.608	6.1	LOS A	4.8	122.1	0.46	0.63	33.6
East: Newport Way											
1	L2	45	2.0	0.743	16.9	LOS A	11.0	280.2	1.00	1.14	29.5
6	T1	250	2.0	0.743	11.4	LOS A	11.0	280.2	1.00	1.14	29.4
16	R2	545	2.0	0.743	11.5	LOS A	11.0	280.2	0.99	1.07	26.2
Approach		841	2.0	0.743	11.7	LOS B	11.0	280.2	0.99	1.10	27.3
North: Lakemont Blvd											
7	L2	402	4.0	0.336	8.8	LOS A	2.5	65.0	0.60	0.65	28.1
4	T1	370	4.0	0.336	2.9	LOS A	2.5	65.0	0.59	0.40	32.0
14	R2	130	4.0	0.336	3.3	LOS A	2.5	65.0	0.49	0.43	31.5
Approach		902	4.0	0.336	5.6	LOS A	2.5	65.0	0.58	0.52	30.0
West: Newport Way											
5	L2	39	4.0	0.171	12.3	LOS A	0.7	18.2	0.59	0.70	32.2
2	T1	79	4.0	0.171	6.5	LOS A	0.7	18.2	0.59	0.70	31.7
12	R2	13	4.0	0.171	6.3	LOS A	0.7	18.2	0.59	0.70	34.2
Approach		132	4.0	0.171	8.2	LOS A	0.7	18.2	0.59	0.70	32.1
All Vehicles		2919	2.4	0.743	7.7	LOS A	11.0	280.2	0.66	0.73	30.4

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

MOVEMENT SUMMARY

Site: 10 [Newport and Lakemont PM]

Roundabout

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Queue Distance ft	Prop. Queued	Effective Stop Rate per veh	Average Speed mph
South: Lakemont Blvd											
3	L2	11	3.0	0.464	19.6	LOS A	3.5	89.8	0.95	1.04	32.9
8	T1	245	3.0	0.464	13.8	LOS A	3.5	89.8	0.95	1.04	29.7
18	R2	287	3.0	0.177	3.6	LOS A	0.0	0.0	0.00	0.44	37.3
Approach		543	3.0	0.464	8.5	LOS A	3.5	89.8	0.44	0.72	33.3
East: Newport Way											
1	L2	110	0.0	0.219	8.1	LOS A	1.5	37.0	0.51	0.50	32.0
6	T1	68	0.0	0.219	2.7	LOS A	1.5	37.0	0.51	0.50	31.9
16	R2	192	0.0	0.219	3.1	LOS A	1.5	37.0	0.50	0.48	28.3
Approach		370	0.0	0.219	4.5	LOS A	1.5	37.0	0.50	0.49	30.0
North: Lakemont Blvd											
7	L2	1177	1.0	0.767	9.4	LOS A	9.8	246.2	0.74	0.63	27.9
4	T1	1010	1.0	0.767	3.1	LOS A	9.3	234.9	0.68	0.42	31.6
14	R2	167	1.0	0.099	2.5	LOS A	0.5	13.4	0.21	0.34	32.3
Approach		2354	1.0	0.767	6.2	LOS A	9.8	246.2	0.68	0.52	29.7
West: Newport Way											
5	L2	40	3.0	0.897	55.6	LOS D	8.9	228.9	0.98	1.40	20.3
2	T1	227	3.0	0.897	49.8	LOS D	8.9	228.9	0.98	1.40	20.1
12	R2	1	3.0	0.897	49.7	LOS D	8.9	228.9	0.98	1.40	21.1
Approach		268	3.0	0.897	50.7	LOS D	8.9	228.9	0.98	1.40	20.2
All Vehicles		3535	1.4	0.897	9.8	LOS A	9.8	246.2	0.65	0.61	29.2

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

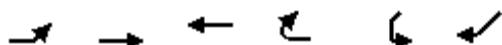
SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Lane Configurations	↑	↑↑	↑				
Traffic Volume (vph)	414	145	763	46	0	0	
Future Volume (vph)	414	145	763	46	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	125			0	0	0	
Storage Lanes	1			0	0	0	
Taper Length (ft)	75				25		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	
Ped Bike Factor				1.00			
Fr _t			0.992				
Flt Protected	0.950						
Satd. Flow (prot)	1770	3539	1846	0	0	0	
Flt Permitted	0.173						
Satd. Flow (perm)	322	3539	1846	0	0	0	
Right Turn on Red				Yes		Yes	
Satd. Flow (RTOR)			4				
Link Speed (mph)		30	30		30		
Link Distance (ft)		412	232		156		
Travel Time (s)		9.4	5.3		3.5		
Confl. Bikes (#/hr)			2				
Peak Hour Factor	0.90	0.90	0.89	0.89	0.25	0.25	
Heavy Vehicles (%)	2%	2%	2%	2%	0%	0%	
Adj. Flow (vph)	460	161	857	52	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	460	161	909	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		12	12		0		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		10	10		10		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors	1	2	2				
Detector Template							
Leading Detector (ft)	25	166	236				
Trailing Detector (ft)	0	0	0				
Detector 1 Position(ft)	0	0	0				
Detector 1 Size(ft)	25	25	25				
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0	0.0				
Detector 2 Position(ft)		160	230				
Detector 2 Size(ft)		6	6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)	0.0	0.0					



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Turn Type	D.P+P	NA	NA				
Protected Phases	1	6	2				8
Permitted Phases	2						
Detector Phase	1	6	2				
Switch Phase							
Minimum Initial (s)	4.0	4.0	4.0				5.0
Minimum Split (s)	10.0	20.0	20.0				30.0
Total Split (s)	20.0	90.0	70.0				30.0
Total Split (%)	16.7%	75.0%	58.3%				25%
Maximum Green (s)	14.9	84.9	64.9				26.0
Yellow Time (s)	4.1	4.1	4.1				3.0
All-Red Time (s)	1.0	1.0	1.0				1.0
Lost Time Adjust (s)	0.0	0.0	0.0				
Total Lost Time (s)	5.1	5.1	5.1				
Lead/Lag	Lead		Lag				
Lead-Lag Optimize?	Yes		Yes				
Vehicle Extension (s)	2.0	2.0	2.0				2.0
Minimum Gap (s)	2.0	2.0	2.0				2.0
Time Before Reduce (s)	0.0	0.0	0.0				0.0
Time To Reduce (s)	5.0	5.0	5.0				5.0
Recall Mode	None	None	None				None
Walk Time (s)							5.0
Flash Dont Walk (s)							21.0
Pedestrian Calls (#/hr)							0
Act Effct Green (s)	59.3	69.8	44.1				
Actuated g/C Ratio	0.85	1.00	0.63				
v/c Ratio	0.78	0.05	0.78				
Control Delay	23.6	0.0	14.3				
Queue Delay	0.0	0.0	0.0				
Total Delay	23.6	0.0	14.3				
LOS	C	A	B				
Approach Delay		17.5	14.3				
Approach LOS		B	B				

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 69.8

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.78

Intersection Signal Delay: 15.6

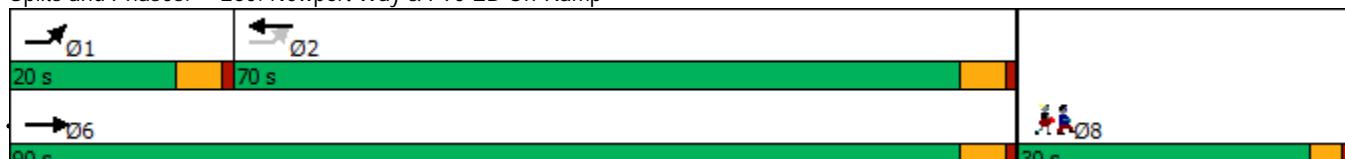
Intersection LOS: B

Intersection Capacity Utilization 74.4%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 260: Newport Way & I-90 EB On-Ramp



Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Lane Configurations	↑	↑↑	↑				
Traffic Volume (vph)	667	607	273	27	0	0	
Future Volume (vph)	667	607	273	27	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	125			0	0	0	
Storage Lanes	1			0	0	0	
Taper Length (ft)	75				25		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	
Fr _t			0.988				
Flt Protected	0.950						
Satd. Flow (prot)	1787	3574	1859	0	0	0	
Flt Permitted	0.412						
Satd. Flow (perm)	775	3574	1859	0	0	0	
Right Turn on Red				Yes		Yes	
Satd. Flow (RTOR)			6				
Link Speed (mph)		30	30		30		
Link Distance (ft)		412	232		156		
Travel Time (s)		9.4	5.3		3.5		
Peak Hour Factor	0.88	0.88	0.78	0.78	0.25	0.25	
Heavy Vehicles (%)	1%	1%	1%	1%	0%	0%	
Adj. Flow (vph)	758	690	350	35	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	758	690	385	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		12	12		0		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		10	10		10		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors	1	2	2				
Detector Template							
Leading Detector (ft)	25	166	236				
Trailing Detector (ft)	0	0	0				
Detector 1 Position(ft)	0	0	0				
Detector 1 Size(ft)	25	25	25				
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0	0.0				
Detector 2 Position(ft)		160	230				
Detector 2 Size(ft)		6	6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type	D.P+P	NA	NA				
Protected Phases	1	6	2				8

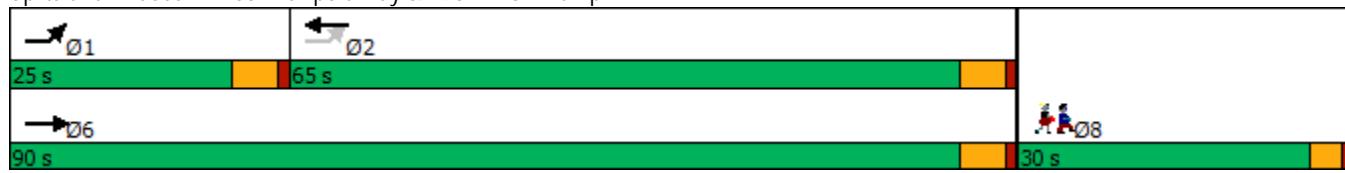


Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Permitted Phases	2						
Detector Phase	1	6	2				
Switch Phase							
Minimum Initial (s)	4.0	4.0	4.0				5.0
Minimum Split (s)	10.0	20.0	20.0				30.0
Total Split (s)	25.0	90.0	65.0				30.0
Total Split (%)	20.8%	75.0%	54.2%				25%
Maximum Green (s)	19.9	84.9	59.9				26.0
Yellow Time (s)	4.1	4.1	4.1				3.0
All-Red Time (s)	1.0	1.0	1.0				1.0
Lost Time Adjust (s)	0.0	0.0	0.0				
Total Lost Time (s)	5.1	5.1	5.1				
Lead/Lag	Lead		Lag				
Lead-Lag Optimize?	Yes		Yes				
Vehicle Extension (s)	2.0	2.0	2.0				2.0
Minimum Gap (s)	2.0	2.0	2.0				2.0
Time Before Reduce (s)	0.0	0.0	0.0				0.0
Time To Reduce (s)	5.0	5.0	5.0				5.0
Recall Mode	None	None	None				None
Walk Time (s)							5.0
Flash Dont Walk (s)							21.0
Pedestrian Calls (#/hr)							0
Act Effct Green (s)	33.6	43.9	13.6				
Actuated g/C Ratio	0.77	1.00	0.31				
v/c Ratio	0.72	0.19	0.66				
Control Delay	9.2	0.1	18.9				
Queue Delay	0.0	0.0	0.0				
Total Delay	9.2	0.1	18.9				
LOS	A	A	B				
Approach Delay		4.9	18.9				
Approach LOS		A	B				

Intersection Summary

Area Type:	Other
Cycle Length:	120
Actuated Cycle Length:	43.9
Natural Cycle:	90
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.72
Intersection Signal Delay:	7.8
Intersection LOS:	A
Intersection Capacity Utilization	61.5%
ICU Level of Service:	B
Analysis Period (min)	15

Splits and Phases: 260: Newport Way & I-90 EB On-Ramp



Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Lane Configurations	↑	↑↑	↑				
Traffic Volume (vph)	600	170	730	40	0	0	
Future Volume (vph)	600	170	730	40	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	125			0	0	0	
Storage Lanes	1			0	0	0	
Taper Length (ft)	75				25		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	
Ped Bike Factor				1.00			
Fr _t			0.993				
Flt Protected	0.950						
Satd. Flow (prot)	1770	3539	1848	0	0	0	
Flt Permitted	0.179						
Satd. Flow (perm)	333	3539	1848	0	0	0	
Right Turn on Red				Yes		Yes	
Satd. Flow (RTOR)			4				
Link Speed (mph)		30	30		30		
Link Distance (ft)		412	232		156		
Travel Time (s)		9.4	5.3		3.5		
Confl. Bikes (#/hr)			2				
Peak Hour Factor	0.90	0.90	0.89	0.89	0.25	0.25	
Heavy Vehicles (%)	2%	2%	2%	2%	0%	0%	
Adj. Flow (vph)	667	189	820	45	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	667	189	865	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		12	12		0		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		10	10		10		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors	1	2	2				
Detector Template							
Leading Detector (ft)	25	166	236				
Trailing Detector (ft)	0	0	0				
Detector 1 Position(ft)	0	0	0				
Detector 1 Size(ft)	25	25	25				
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0	0.0				
Detector 2 Position(ft)		160	230				
Detector 2 Size(ft)		6	6				
Detector 2 Type		Cl+Ex	Cl+Ex				
Detector 2 Channel							
Detector 2 Extend (s)	0.0	0.0					

Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Turn Type	D.P+P	NA	NA				
Protected Phases	1	6	2				8
Permitted Phases	2						
Detector Phase	1	6	2				
Switch Phase							
Minimum Initial (s)	4.0	4.0	4.0				5.0
Minimum Split (s)	10.0	20.0	20.0				30.0
Total Split (s)	20.0	90.0	70.0				30.0
Total Split (%)	16.7%	75.0%	58.3%				25%
Maximum Green (s)	14.9	84.9	64.9				26.0
Yellow Time (s)	4.1	4.1	4.1				3.0
All-Red Time (s)	1.0	1.0	1.0				1.0
Lost Time Adjust (s)	0.0	0.0	0.0				
Total Lost Time (s)	5.1	5.1	5.1				
Lead/Lag	Lead		Lag				
Lead-Lag Optimize?	Yes		Yes				
Vehicle Extension (s)	2.0	2.0	2.0				2.0
Minimum Gap (s)	2.0	2.0	2.0				2.0
Time Before Reduce (s)	0.0	0.0	0.0				0.0
Time To Reduce (s)	5.0	5.0	5.0				5.0
Recall Mode	None	None	None				None
Walk Time (s)							5.0
Flash Dont Walk (s)							21.0
Pedestrian Calls (#/hr)							0
Act Effct Green (s)	53.8	64.3	38.6				
Actuated g/C Ratio	0.84	1.00	0.60				
v/c Ratio	1.08	0.05	0.78				
Control Delay	77.6	0.0	14.8				
Queue Delay	0.0	0.0	0.0				
Total Delay	77.6	0.0	14.8				
LOS	E	A	B				
Approach Delay		60.5	14.8				
Approach LOS		E	B				

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 64.3

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.08

Intersection Signal Delay: 37.5

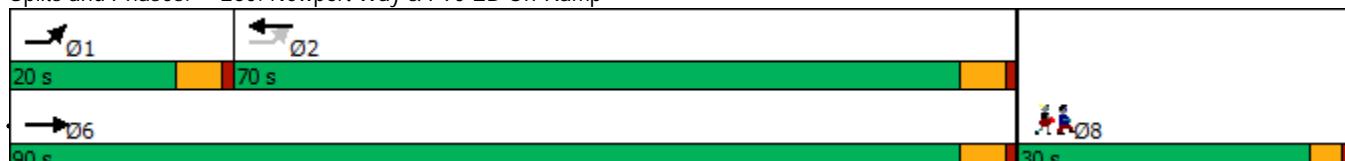
Intersection LOS: D

Intersection Capacity Utilization 82.6%

ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 260: Newport Way & I-90 EB On-Ramp



Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Lane Configurations	↑	↑↑	↑				
Traffic Volume (vph)	1160	570	230	30	0	0	
Future Volume (vph)	1160	570	230	30	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Storage Length (ft)	125			0	0	0	
Storage Lanes	1			0	0	0	
Taper Length (ft)	75				25		
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	
Frt			0.985				
Flt Protected	0.950						
Satd. Flow (prot)	1787	3574	1853	0	0	0	
Flt Permitted	0.478						
Satd. Flow (perm)	899	3574	1853	0	0	0	
Right Turn on Red				Yes		Yes	
Satd. Flow (RTOR)			8				
Link Speed (mph)		30	30		30		
Link Distance (ft)		412	232		156		
Travel Time (s)		9.4	5.3		3.5		
Peak Hour Factor	0.88	0.88	0.78	0.78	0.25	0.25	
Heavy Vehicles (%)	1%	1%	1%	1%	0%	0%	
Adj. Flow (vph)	1318	648	295	38	0	0	
Shared Lane Traffic (%)							
Lane Group Flow (vph)	1318	648	333	0	0	0	
Enter Blocked Intersection	No	No	No	No	No	No	
Lane Alignment	Left	Left	Left	Right	Left	Right	
Median Width(ft)		12	12		0		
Link Offset(ft)		0	0		0		
Crosswalk Width(ft)		10	10		10		
Two way Left Turn Lane							
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Speed (mph)	15			9	15	9	
Number of Detectors	1	2	2				
Detector Template							
Leading Detector (ft)	25	166	236				
Trailing Detector (ft)	0	0	0				
Detector 1 Position(ft)	0	0	0				
Detector 1 Size(ft)	25	25	25				
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex				
Detector 1 Channel							
Detector 1 Extend (s)	0.0	0.0	0.0				
Detector 1 Queue (s)	0.0	0.0	0.0				
Detector 1 Delay (s)	0.0	0.0	0.0				
Detector 2 Position(ft)		160	230				
Detector 2 Size(ft)		6	6				
Detector 2 Type	Cl+Ex	Cl+Ex					
Detector 2 Channel							
Detector 2 Extend (s)		0.0	0.0				
Turn Type	D.P+P	NA	NA				
Protected Phases	1	6	2				8

Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019

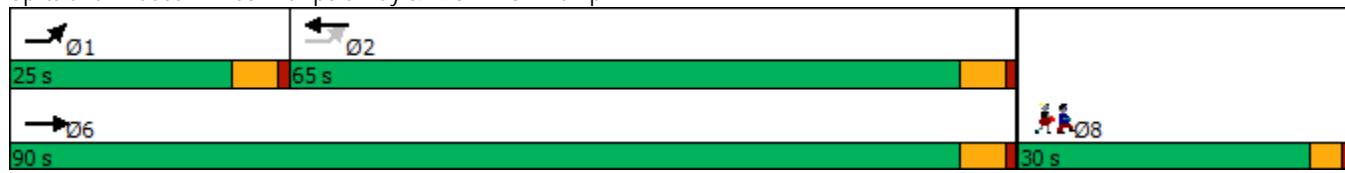


Lane Group	EBL	EBT	WBT	WBR	SWL	SWR	Ø8
Permitted Phases	2						
Detector Phase	1	6	2				
Switch Phase							
Minimum Initial (s)	4.0	4.0	4.0				5.0
Minimum Split (s)	10.0	20.0	20.0				30.0
Total Split (s)	25.0	90.0	65.0				30.0
Total Split (%)	20.8%	75.0%	54.2%				25%
Maximum Green (s)	19.9	84.9	59.9				26.0
Yellow Time (s)	4.1	4.1	4.1				3.0
All-Red Time (s)	1.0	1.0	1.0				1.0
Lost Time Adjust (s)	0.0	0.0	0.0				
Total Lost Time (s)	5.1	5.1	5.1				
Lead/Lag	Lead		Lag				
Lead-Lag Optimize?	Yes		Yes				
Vehicle Extension (s)	2.0	2.0	2.0				2.0
Minimum Gap (s)	2.0	2.0	2.0				2.0
Time Before Reduce (s)	0.0	0.0	0.0				0.0
Time To Reduce (s)	5.0	5.0	5.0				5.0
Recall Mode	None	None	None				None
Walk Time (s)							5.0
Flash Dont Walk (s)							21.0
Pedestrian Calls (#/hr)							0
Act Effct Green (s)	32.0	42.2	12.0				
Actuated g/C Ratio	0.76	1.00	0.28				
v/c Ratio	1.20	0.18	0.63				
Control Delay	111.6	0.1	18.4				
Queue Delay	0.0	0.0	0.0				
Total Delay	111.6	0.1	18.4				
LOS	F	A	B				
Approach Delay		74.8	18.4				
Approach LOS		E	B				

Intersection Summary

Area Type:	Other
Cycle Length:	120
Actuated Cycle Length:	42.2
Natural Cycle:	150
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	1.20
Intersection Signal Delay:	66.6
Intersection LOS:	E
Intersection Capacity Utilization:	86.7%
ICU Level of Service:	E
Analysis Period (min):	15

Splits and Phases: 260: Newport Way & I-90 EB On-Ramp



Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	↑↑	↑	↑			
Traffic Volume (vph)	600	170	730	40	0	0
Future Volume (vph)	600	170	730	40	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	125			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	75				25	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor				1.00		
Fr _t			0.993			
Flt Protected	0.950					
Satd. Flow (prot)	3433	1863	1848	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3433	1863	1848	0	0	0
Right Turn on Red				Yes		Yes
Satd. Flow (RTOR)			6			
Link Speed (mph)		30	30		30	
Link Distance (ft)		412	232		156	
Travel Time (s)		9.4	5.3		3.5	
Confl. Bikes (#/hr)			2			
Peak Hour Factor	0.90	0.90	0.89	0.89	0.88	0.88
Adj. Flow (vph)	667	189	820	45	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	667	189	865	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		24	24		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		10	10		10	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Number of Detectors	1	2	2			
Detector Template						
Leading Detector (ft)	25	166	236			
Trailing Detector (ft)	0	0	0			
Detector 1 Position(ft)	0	0	0			
Detector 1 Size(ft)	25	25	25			
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex			
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0	0.0			
Detector 2 Position(ft)		160	230			
Detector 2 Size(ft)		6	6			
Detector 2 Type	Cl+Ex	Cl+Ex				
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA			

Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019

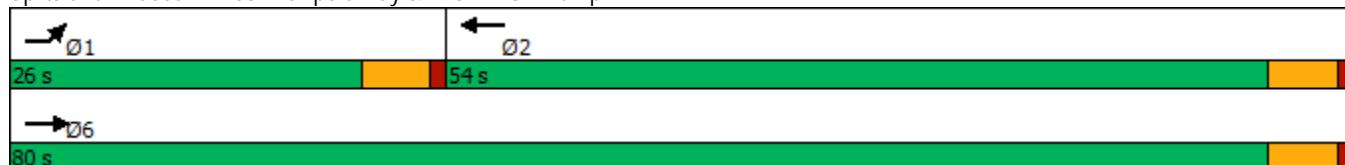


Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Protected Phases	1	6	2			
Permitted Phases						
Detector Phase	1	6	2			
Switch Phase						
Minimum Initial (s)	4.0	4.0	4.0			
Minimum Split (s)	10.0	20.0	20.0			
Total Split (s)	26.0	80.0	54.0			
Total Split (%)	32.5%	100.0%	67.5%			
Maximum Green (s)	20.9	74.9	48.9			
Yellow Time (s)	4.1	4.1	4.1			
All-Red Time (s)	1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0	0.0			
Total Lost Time (s)	5.1	5.1	5.1			
Lead/Lag	Lead		Lag			
Lead-Lag Optimize?	Yes		Yes			
Vehicle Extension (s)	2.0	2.0	2.0			
Minimum Gap (s)	2.0	2.0	2.0			
Time Before Reduce (s)	0.0	0.0	0.0			
Time To Reduce (s)	5.0	5.0	5.0			
Recall Mode	None	None	None			
Act Effct Green (s)	16.4	64.2	36.9			
Actuated g/C Ratio	0.26	1.00	0.57			
v/c Ratio	0.76	0.10	0.81			
Control Delay	30.0	0.1	18.3			
Queue Delay	0.0	0.0	0.0			
Total Delay	30.0	0.1	18.3			
LOS	C	A	B			
Approach Delay		23.4	18.3			
Approach LOS		C	B			

Intersection Summary

Area Type:	Other
Cycle Length:	80
Actuated Cycle Length:	64.2
Natural Cycle:	55
Control Type:	Actuated-Uncoordinated
Maximum v/c Ratio:	0.81
Intersection Signal Delay:	20.9
Intersection LOS:	C
Intersection Capacity Utilization	66.5%
ICU Level of Service	C
Analysis Period (min)	15

Splits and Phases: 260: Newport Way & I-90 EB On-Ramp



Lanes, Volumes, Timings
260: Newport Way & I-90 EB On-Ramp

05/28/2019



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Lane Configurations	↑↑	↑	↑			
Traffic Volume (vph)	1160	570	230	30	0	0
Future Volume (vph)	1160	570	230	30	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	125			0	0	0
Storage Lanes	1			0	0	0
Taper Length (ft)	75				25	
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00
Fr _t			0.985			
Flt Protected	0.950					
Satd. Flow (prot)	3467	1881	1853	0	0	0
Flt Permitted	0.950					
Satd. Flow (perm)	3467	1881	1853	0	0	0
Right Turn on Red			Yes		Yes	
Satd. Flow (RTOR)			11			
Link Speed (mph)		30	30		30	
Link Distance (ft)		412	232		156	
Travel Time (s)		9.4	5.3		3.5	
Peak Hour Factor	0.88	0.88	0.78	0.78	0.25	0.25
Heavy Vehicles (%)	1%	1%	1%	1%	0%	0%
Adj. Flow (vph)	1318	648	295	38	0	0
Shared Lane Traffic (%)						
Lane Group Flow (vph)	1318	648	333	0	0	0
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Left	Left	Right	Left	Right
Median Width(ft)		24	24		0	
Link Offset(ft)		0	0		0	
Crosswalk Width(ft)		10	10		10	
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15			9	15	9
Number of Detectors	1	2	2			
Detector Template						
Leading Detector (ft)	25	166	236			
Trailing Detector (ft)	0	0	0			
Detector 1 Position(ft)	0	0	0			
Detector 1 Size(ft)	25	25	25			
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex			
Detector 1 Channel						
Detector 1 Extend (s)	0.0	0.0	0.0			
Detector 1 Queue (s)	0.0	0.0	0.0			
Detector 1 Delay (s)	0.0	0.0	0.0			
Detector 2 Position(ft)		160	230			
Detector 2 Size(ft)		6	6			
Detector 2 Type	Cl+Ex	Cl+Ex				
Detector 2 Channel						
Detector 2 Extend (s)		0.0	0.0			
Turn Type	Prot	NA	NA			
Protected Phases	1	6	2			



Lane Group	EBL	EBT	WBT	WBR	SWL	SWR
Permitted Phases						
Detector Phase	1	6	2			
Switch Phase						
Minimum Initial (s)	4.0	4.0	4.0			
Minimum Split (s)	10.0	20.0	20.0			
Total Split (s)	39.0	65.0	26.0			
Total Split (%)	60.0%	100.0%	40.0%			
Maximum Green (s)	33.9	59.9	20.9			
Yellow Time (s)	4.1	4.1	4.1			
All-Red Time (s)	1.0	1.0	1.0			
Lost Time Adjust (s)	0.0	0.0	0.0			
Total Lost Time (s)	5.1	5.1	5.1			
Lead/Lag	Lead	Lag				
Lead-Lag Optimize?	Yes	Yes				
Vehicle Extension (s)	2.0	2.0	2.0			
Minimum Gap (s)	2.0	2.0	2.0			
Time Before Reduce (s)	0.0	0.0	0.0			
Time To Reduce (s)	5.0	5.0	5.0			
Recall Mode	None	Min	None			
Act Effect Green (s)	22.9	47.1	13.2			
Actuated g/C Ratio	0.49	1.00	0.28			
v/c Ratio	0.78	0.34	0.63			
Control Delay	14.2	0.5	21.4			
Queue Delay	0.0	0.0	0.0			
Total Delay	14.2	0.5	21.4			
LOS	B	A	C			
Approach Delay		9.7	21.4			
Approach LOS		A	C			

Intersection Summary

Area Type: Other

Cycle Length: 65

Actuated Cycle Length: 47.1

Natural Cycle: 50

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.78

Intersection Signal Delay: 11.4

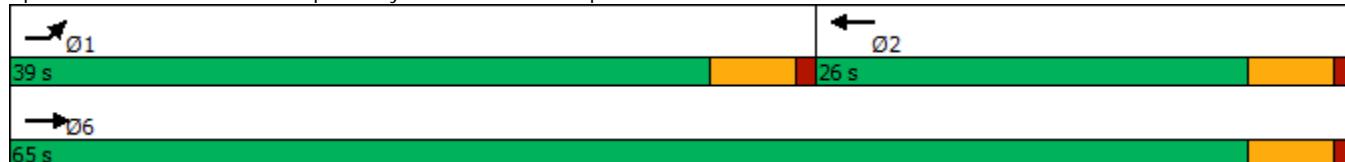
Intersection LOS: B

Intersection Capacity Utilization 55.5%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 260: Newport Way & I-90 EB On-Ramp



MOVEMENT SUMMARY

Site: 11 [Newport and I-90 AM]

Roundabout

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Queue Distance ft	Prop. Queued	Effective Stop Rate per veh	Average Speed mph
East: Newport Way											
6	T1	820	2.0	0.901	12.0	LOS D	11.9	301.3	0.88	1.25	26.8
16	R2	45	2.0	0.901	12.4	LOS D	11.9	301.3	0.88	1.25	26.2
Approach		865	2.0	0.901	12.0	LOS B	11.9	301.3	0.88	1.25	26.8
West: Newport Way											
5	L2	667	2.0	0.269	7.1	LOS A	0.0	0.0	0.00	0.54	29.2
2	T1	189	2.0	0.269	1.5	LOS A	0.0	0.0	0.00	0.47	29.3
Approach		856	2.0	0.269	5.9	LOS A	0.0	0.0	0.00	0.53	29.2
All Vehicles		1721	2.0	0.901	8.9	LOS A	11.9	301.3	0.44	0.89	28.0

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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MOVEMENT SUMMARY

Site: 11 [Newport and I-90 PM]

Roundabout

Movement Performance - Vehicles											
Mov ID	OD Mov	Demand Flows Total veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Queue Distance ft	Prop. Queued	Effective Stop Rate per veh	Average Speed mph
East: Newport Way											
6	T1	295	1.0	0.436	7.4	LOS A	2.2	56.0	0.69	0.82	28.4
16	R2	38	1.0	0.436	7.8	LOS A	2.2	56.0	0.69	0.82	27.6
Approach		333	1.0	0.436	7.4	LOS A	2.2	56.0	0.69	0.82	28.3
West: Newport Way											
5	L2	1318	1.0	0.611	7.1	LOS A	0.0	0.0	0.00	0.53	29.3
2	T1	648	1.0	0.611	1.5	LOS A	0.0	0.0	0.00	0.39	29.7
Approach		1966	1.0	0.611	5.3	LOS A	0.0	0.0	0.00	0.48	29.4
All Vehicles		2299	1.0	0.611	5.6	LOS A	2.2	56.0	0.10	0.53	29.2

Site Level of Service (LOS) Method: Delay & Degree of Saturation (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

Intersection and Approach LOS values are based on average delay for all movements (v/c not used).

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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APPENDIX E: OPINION OF PROBABLE CONSTRUCTION COSTS

City of Bellevue
Lakemont Blvd & Newport Way Alternative 1
Modified Signal

Preliminary Opinion of Probable Construction Cost

<u>ITEM</u>	<u>QTY</u>	<u>UNIT</u>	<u>UNIT COST</u>	<u>COST</u>
PREPARATION				
Mobilization	1	LS	\$195,000	\$195,000
Construction Surveying	1	LS	\$42,000	\$42,000
Clearing & Grubbing	0.6	AC	\$50,000	\$30,000
TRAFFIC CONTROL				
Traffic Control	1	LS	\$310,000	\$310,000
GRADING				
Roadway Excavation Incl. Haul	1,470	CY	\$50	\$73,500
Gravel Borrow	1,180	TN	\$30	\$35,400
ROADWAY SECTION				
Planing Bituminous Pavement	6,070	SY	\$8	\$48,560
HMA Cl. 1/2 in. PG 58H-22	1,050	TN	\$130	\$136,500
Crushed Surfacing Base Course	1,660	TN	\$40	\$66,400
STORM DRAINAGE				
Stormwater Improvements	1	LS	\$110,000	\$110,000
EROSION CONTROL				
TESC	1	LS	\$37,000	\$37,000
CURBING				
Cement Conc. Curbs	810	LF	\$40	\$32,400
STRIPING & SIGNING				
Channelization and Signing	1	LS	\$16,320	\$16,320
TRAFFIC SIGNAL				
Traffic Signal	1	LS	\$600,000	\$600,000
SIDEWALK & RAMPS				
Cement Conc. Sidewalk	1,230	SY	\$75	\$92,250
Cement Concrete Curb Ramps	6	EA	\$3,500	\$21,000
OTHER ITEMS				
Landscaping & Irrigation	820	SY	\$125	\$102,500
Retaining Walls	1,000	SF	\$150	\$150,000
Miscellaneous/Unknown Costs	1	LS	\$420,000	\$420,000
			Subtotal	\$2,520,000
			Contincency (30%)	\$760,000
			Total	\$3,300,000

City of Bellevue
Lakemont Blvd & Newport Way Alternative 2
Roundabout

Preliminary Opinion of Probable Construction Cost

<u>ITEM</u>	<u>QTY</u>	<u>UNIT</u>	<u>UNIT COST</u>	<u>COST</u>
PREPARATION				
Mobilization	1	LS	\$500,000	\$500,000
Construction Surveying	1	LS	\$150,000	\$150,000
Clearing & Grubbing	1.1	AC	\$50,000	\$55,000
TRAFFIC CONTROL				
Traffic Control	1	LS	\$800,000	\$800,000
ROADWAY PREPARATION				
Roadway Excavation Incl. Haul	11,200	CY	\$50	\$560,000
Gravel Borrow	7,100	TN	\$30	\$213,000
ROADWAY SECTION				
HMA Cl. 1/2 in. PG 58H-22	7,350	TN	\$130	\$955,500
Crushed Surfacing Base Course	7,770	TN	\$40	\$310,800
STORM DRAINAGE				
Stormwater Improvements	1	LS	\$532,000	\$532,000
EROSION CONTROL				
TESC	1	LS	\$84,000	\$84,000
CURBING				
Cement Conc. Curbs	7,500	LF	\$40	\$300,000
STRIPING & SIGNING				
Channelization and Signing	1	LS	\$110,000	\$110,000
ILLUMINATION				
Illumination System	1	LS	\$200,000	\$200,000
SIDEWALK & RAMPS				
Cement Conc. Sidewalk	2,800	SY	\$75	\$210,000
Cement Concrete Curb Ramps	13	EA	\$3,500	\$45,500
OTHER ITEMS				
Landscaping & Irrigation	3,500	SY	\$125	\$437,500
Retaining Walls	7,500	SF	\$150	\$1,125,000
Miscellaneous/Unknown Costs	1	LS	\$2,000,000	\$2,000,000
			Subtotal	\$8,590,000
			Contingency (30%)	\$2,580,000
			Total	\$11,200,000

APPENDIX F: PUBLIC COMMENTS



COMMENT SHEET

Intersection Study on Lakemont Blvd

Open House – February 20, 2019



Your input is valuable. Please add your comments, questions, concerns, and ideas for one or more of the study intersections and leave this sheet with us tonight.

Lakemont Blvd and Forest Drive SE

No traffic light! a traffic light will be too disruptive for traffic on Lakemont

Lakemont Blvd SE and SE Newport Way

Like the roundabout solution to keep traffic moving

I-90 Eastbound On-ramp on SE Newport Way

The _____
de your input.

Name _____

Contact information (phone, email and/or address):

For more information about the project or to submit comments online, visit:

BellevueWa.gov/transportationlevy

You can also contact:

Jun An, P.E., Project Manager | 425-452-4230 | jan@BellevueWa.gov



COMMENT SHEET

Intersection Study on Lakemont Blvd

Open House – February 20, 2019



Your input is valuable. Please add your comments, questions, concerns, and ideas for one or more of the study intersections and leave this sheet with us tonight.

Lakemont Blvd and Forest Drive SE

At a minimum a left turn lane from Lakemont N to Forest would be a big help.

The traffic light would be a bigger help ~~for~~ for those turning from Forest onto Lakemont - Lower priority but still helpful.

Lakemont Blvd SE and SE Newport Way

No strong opinion on this one or the one below - traffic circles seem to make sense but disruptions from removal of existing lights and installation of circles is something I'm not looking forward to.

I-90 Eastbound On-ramp on SE Newport Way

Thank you for taking the time to provide your input.

Name: _____

Contact info _____ d/or addr.

For more information about the project or to submit comments online, visit:
BellevueWa.gov/transportationlevy

You can also contact:

Jun An, P.E., Project Manager | 425-452-4230 | jan@BellevueWa.gov



COMMENT SHEET

Intersection Study on Lakemont Blvd

Open House – February 20, 2019



Your input is valuable. Please add your comments, questions, concerns, and ideas for one or more of the study intersections and leave this sheet with us tonight.

Lakemont Blvd and Forest Drive SE

Left hand turn from Lnt. Blvd Northbound into FD will dramatically reduce backup on FD/Lnt Blvd junction in the AFTERNOON. TRAFFIC SIGNAL NOT NEEDED.

Lakemont Blvd SE and SE Newport Way

ROUNDABOUT is a skin dump

I-90 Eastbound On-ramp on SE Newport Way

" " SAME

SE Cougar Mt Way /Lakemont signal: Real issues -NOT NEEDED except 7:00-10:00am & 3:30-6:30pm M-F. WEEKEND NOT NEEDED AT ALL CARS turning north on Lakemont from Cougar Mt. Way trigger turn signal evenif no oncomers traffic forcing a stop.

Thank you for taking the time to provide your input.

Name: _____

Contact info: _____

submit comments online, visit:

BellevueWa.gov/transportationlevy

You can also contact:

Jun An, P.E., Project Manager | 425-452-4230 | jan@BellevueWa.gov



COMMENT SHEET
Intersection Study on Lakemont Blvd
Open House – February 20, 2019



Your input is valuable. Please add your comments, questions, concerns, and ideas for one or more of the study intersections and leave this sheet with us tonight.

Lakemont Blvd and Forest Drive SE

Left turn lane from Northbound Lakemont to Forest is good idea and would help. Just widening Forest to have fairly long right turn lane might be sufficient first step.

Lakemont Blvd SE and SE Newport Way

Double roundabout idea seems best option to me. It is sorely needed.

I-90 Eastbound On-ramp on SE Newport Way

Just making left turn green choice. Signal stay green longer would be a useful 1st step.

Thank you for taking the time to provide your input.

Name: _____

Contact _____
Address _____

For more information about the project or to submit comments online, visit:

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COMMENT SHEET

Intersection Study on Lakemont Blvd

Open House – February 20, 2019



Your input is valuable. Please add your comments, questions, concerns, and ideas for one or more of the study intersections and leave this sheet with us tonight.

Lakemont Blvd and Forest Drive SE

Has the volume of bicycle traffic on Lakemont been addressed? Traffic signal with left turn lane would work best but without left turn would work also

Lakemont Blvd SE and SE Newport Way

Roundabout looks like best option

I-90 Eastbound On-ramp on SE Newport Way

Really like 2 lanes for traffic onto I-90.

Thank you for taking the time to provide your input.

Co.

ddress): _____

For more information about the project or to submit comments online, visit:

BellevueWa.gov/transportationlevy

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